

①	B6686	BRIDGE LAYOUT	30243 39243
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Stations and elevations are in meters. All other dimensions are in millimeters unless otherwise noted.

BENCH MARK: Standard Bronze Benchmark SW corner
of bridge across White River/Beaver Lake
"19WHV reset 1961", Sta. 88+35.985 Offset 440.061
Elev. 347.551 m, N 218,708.917 m E 25,360.030 m.

DESIGN SPECIFICATIONS: AASHTO Standard Specifications for Highway Bridges, (1996 edition) with current interim specifications.

MATERIALS AND STRENGTHS:

Class S(AE) Concrete (superstructure)	$f'c = 28$ MPa
Class S Concrete (substructure)	$f'c = 24$ MPa
Class S (modified) Concrete (drilled shafts)	$f'c = 28$ MPa
Reinf. Steel (AASHTO M31 or M53 GR. 400)	$f_y = 400$ MPa
Structural Steel (AASHTO M270, GR. 345W)	$f_y = 345$ MPa

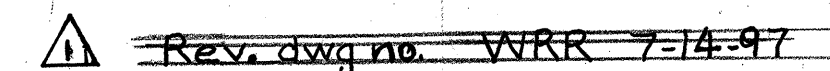
ABUTMENT FOOTING: The footing shall be founded a minimum of 0.3m into the material designated as Hard Chert and Limestone rock formation on the boring legend. The allowable bearing capacity for this rock formation is 1200KPa. The rock at the abutment shall be cored to a depth of 2m below the bottom of the abutment.

STEEL PILING: Piling in End Bent 6 shall be H Piles and shall be driven with an approved air, steam, or diesel hammer to a minimum safe bearing capacity of 448kN per pile and into the material designated as hard gray limestone on the boring legend. Lengths of piling shown are for estimating quantities and for use in determining payment for cut-off and build-up in accordance with the standard specifications. Piles in end bent to be driven after embankment to bottom of cap is in place. Approved pile points shall be used.

EASTBOUND BRIDGE
SHEET 2 OF 2
LAYOUT OF EASTBOUND
BORINGS AND NOTES
US. HWY. 412
ARKANSAS STATE HIGHWAY
COMMISSION
LITTLE ROCK, ARKANSAS

DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted
DESIGNED BY: SLH DATE: 12/96

BRIDGE NO. 86686 DRAWING NO. ~~30223~~ **39243**

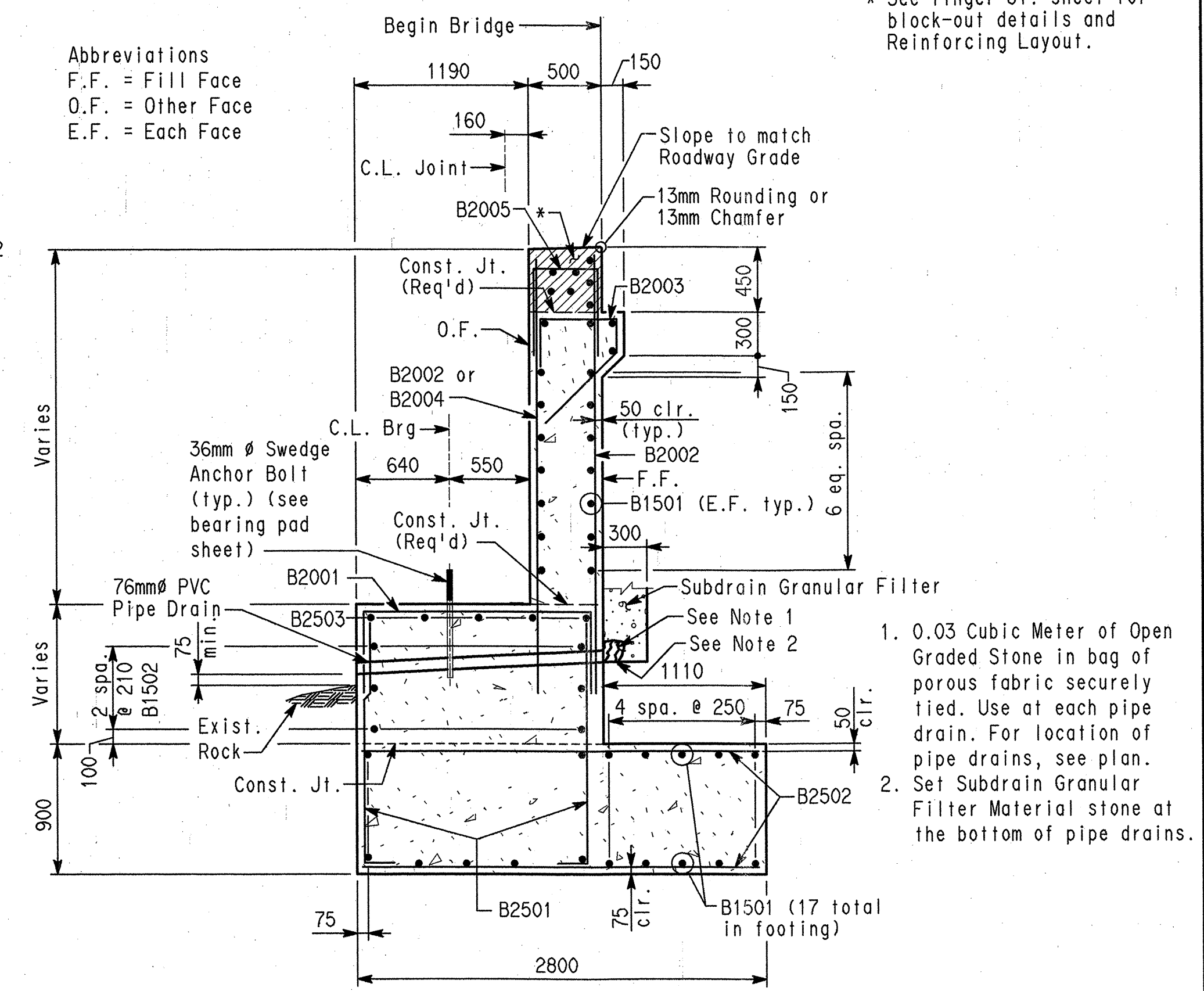
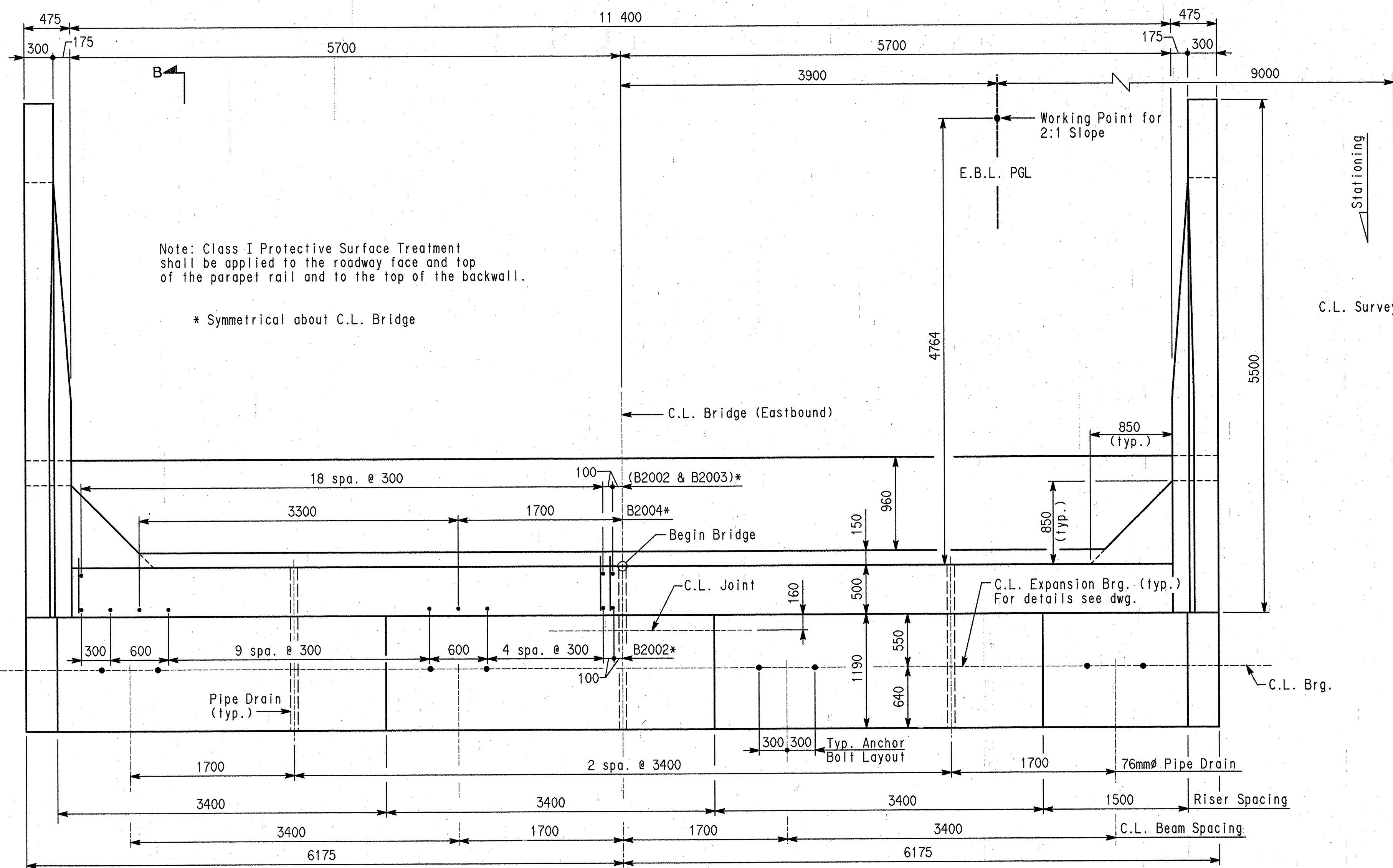


BRIDGE DECK: The concrete bridge deck shall be given a fine finish as specified for final finishing in subsection 802.19 for Class 5 Tined Bridge Roadway Surface Finish.

FILL MATERIAL: Select material as specified in the roadway plans shall be used for the 2:1 end slope at Bent 6.

O-Firm reddish tan fine sandy silty clay with organics
P-Stiff to firm tan silty fine sandy clay
Q-Very soft reddish brown and tan silty clay with organics
R-Hard light gray limestone
S-Topsoil
T-Firm brown silty clay
U-Firm reddish and yellowish brown silty clay
V-Soft brown and tan silty sandy clay
W-Stiff to very stiff brown and tan silty sandy clay
X-Dense brown and tan silty sandy clay
Y-Hard gray limestone

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	Fed Road Dist. No.	State	Fed. and Proj No.	Sheet No.	Total Sheets
7-14-97	7-25-97				Ark.		20	118
				Job No.		040236		
				B6686		BENT 1	30244	33



NOTES

Stations and elevations are in meter (m). All other dimensions are in millimeters (mm) unless otherwise noted.

All concrete shall be class "S" with a minimum 28 day compressive strength $f'_c = 24 \text{ MPa}$. Concrete shall be poured in the dry and all exposed corners to be chamfered 20 unless otherwise noted.

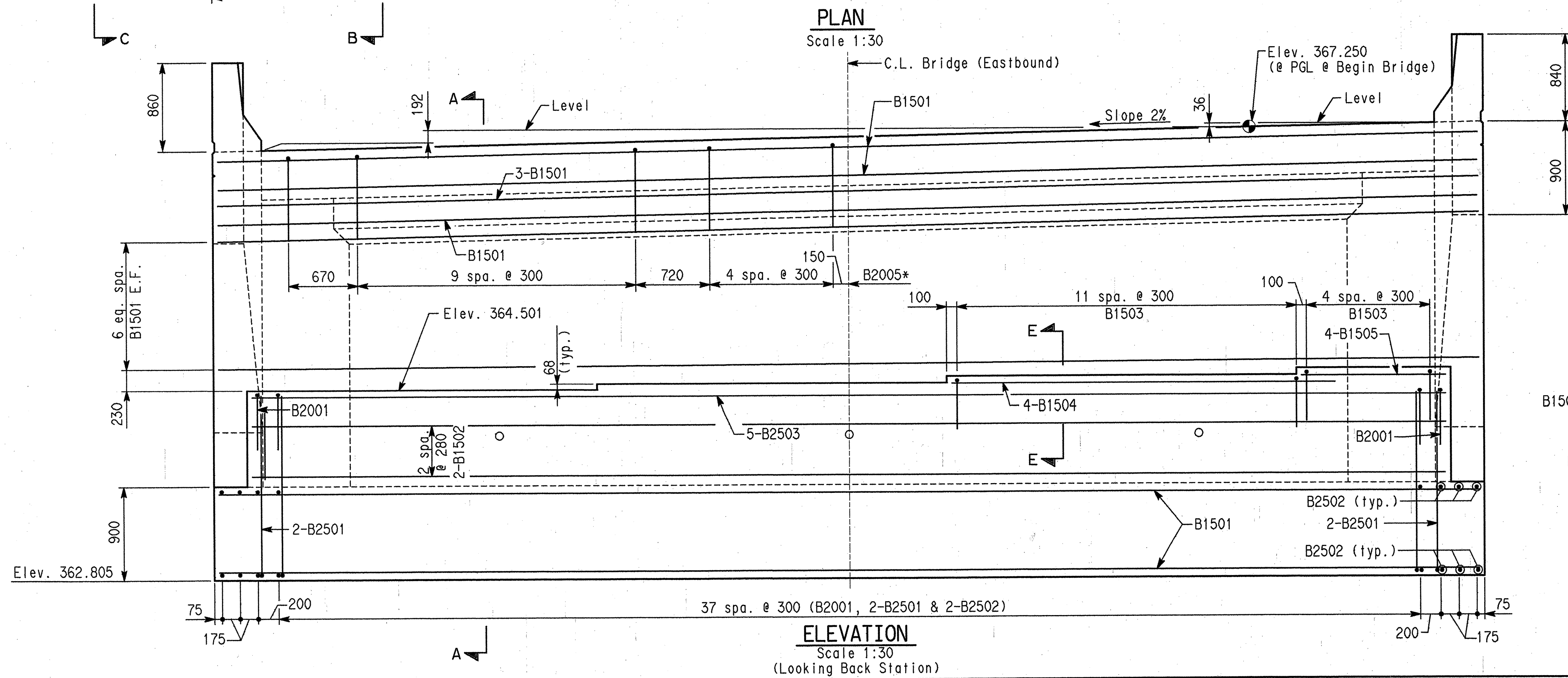
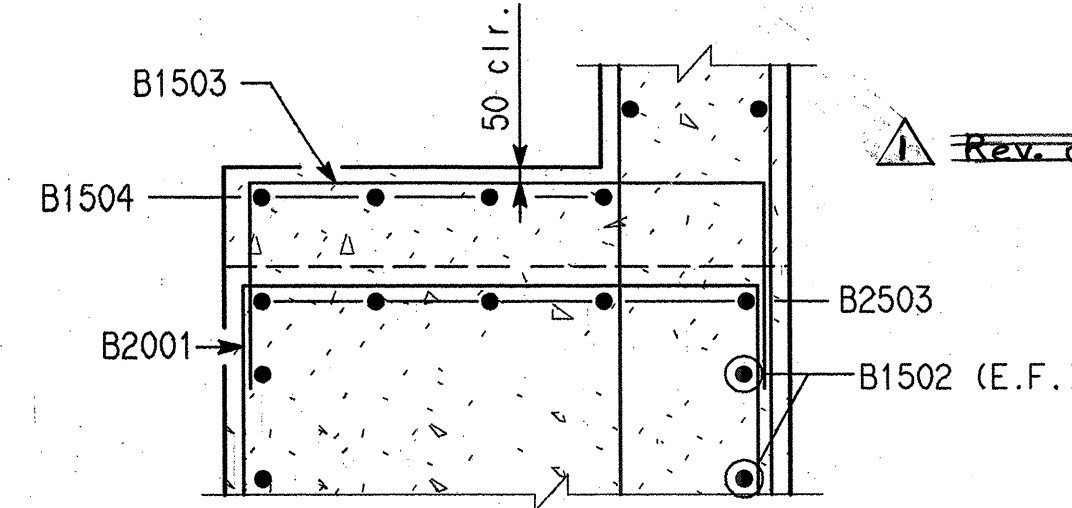
All reinforcing steel shall conform to AASHTO M31M or M53M, Gr. 400 (yield strength = 400 MPa).

Backwall shall not be poured before beams are in place.

Structural steel in end bents (see Finger Joint Detail sheet) shall be AASHTO M270, Gr. 345W and shall be paid for as "Structural Steel in Plate Girder Spans (M270, Gr. 345W)".

If anchor bolts are drilled into cap, top reinforcing bars shall be properly placed to avoid damage.

For additional information see layout.



Arkansas Temporary Permit Number 96-45, Issued 12-15-96.
Signature of Holder *[Signature]* 2/1/97

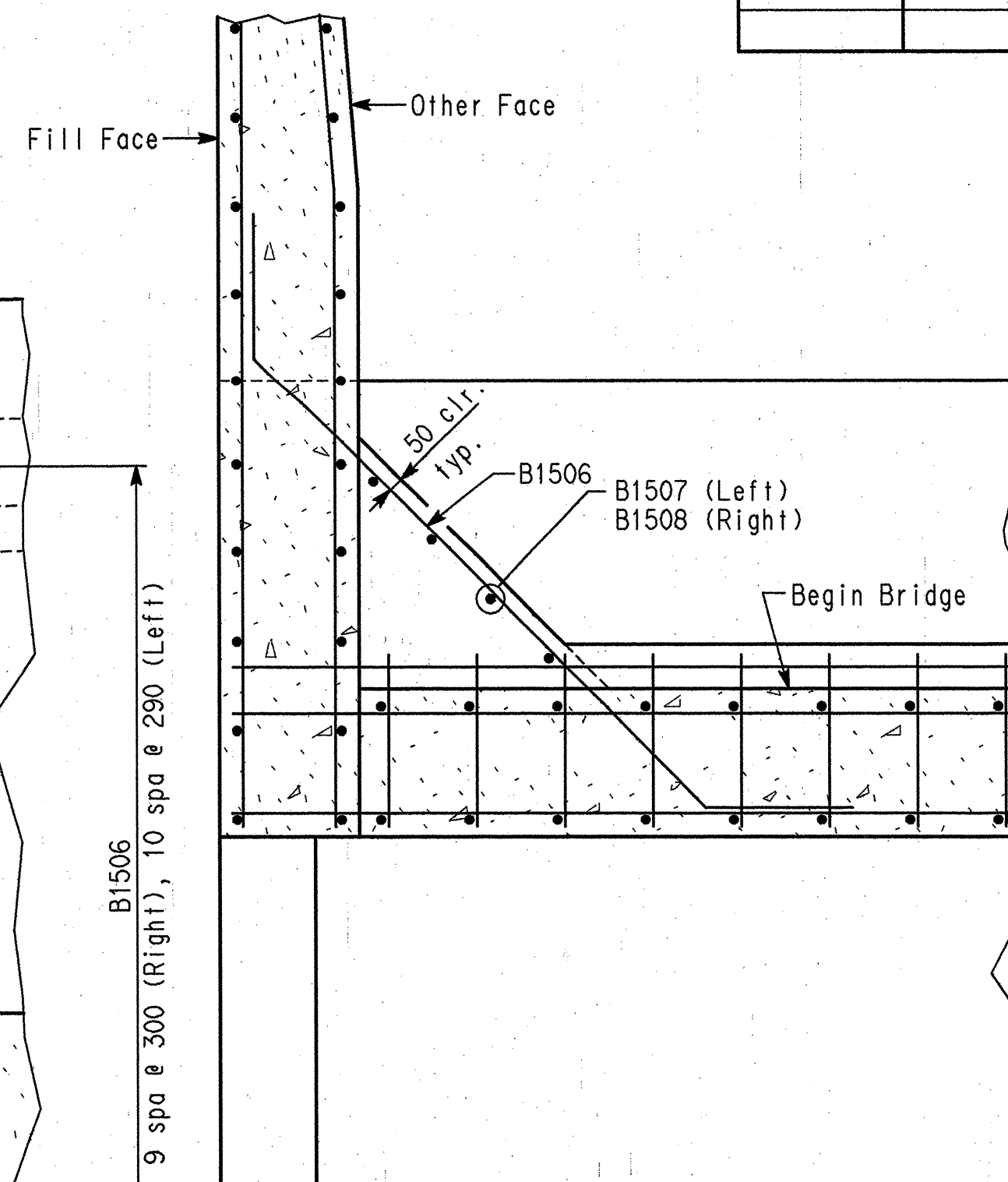
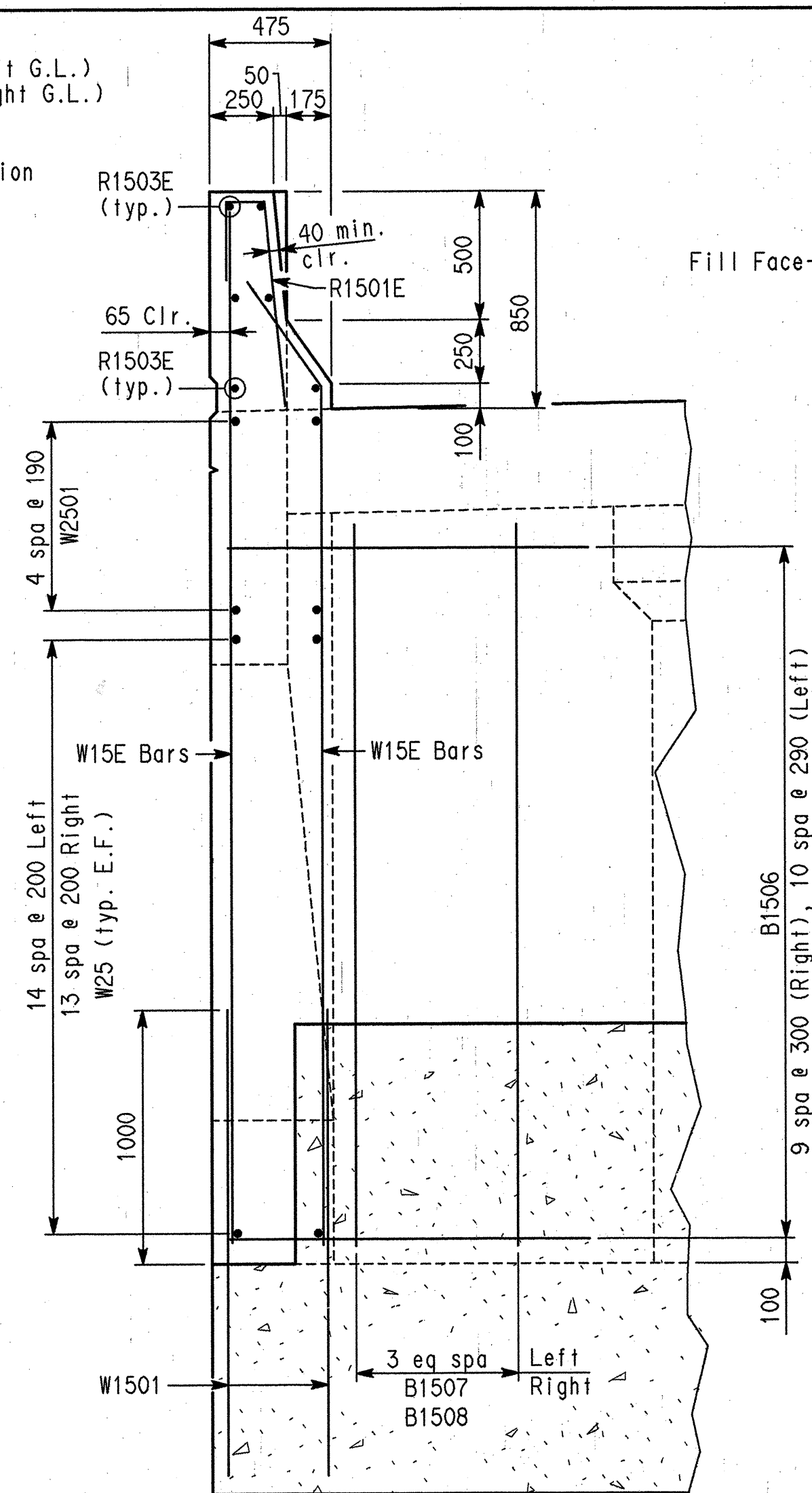
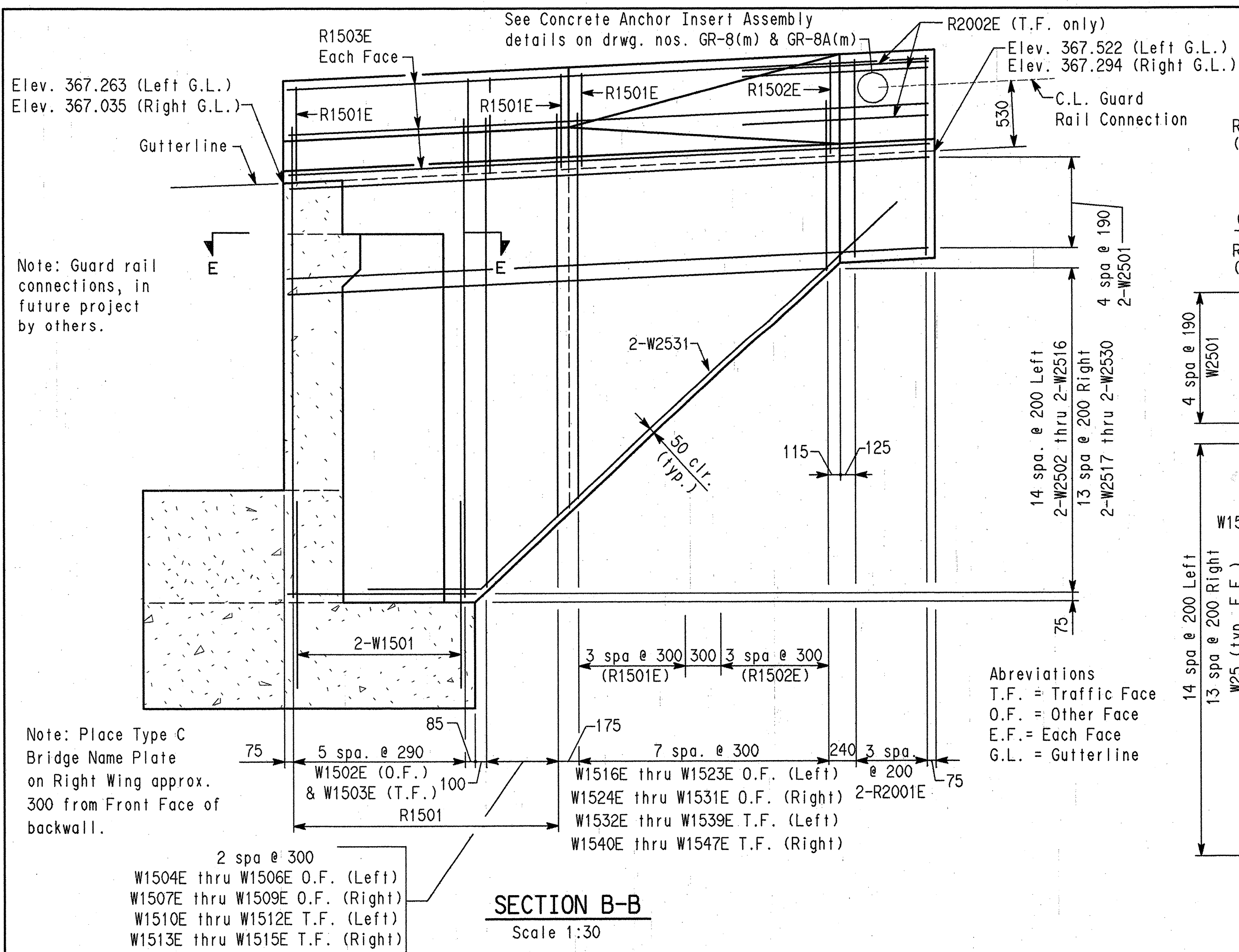
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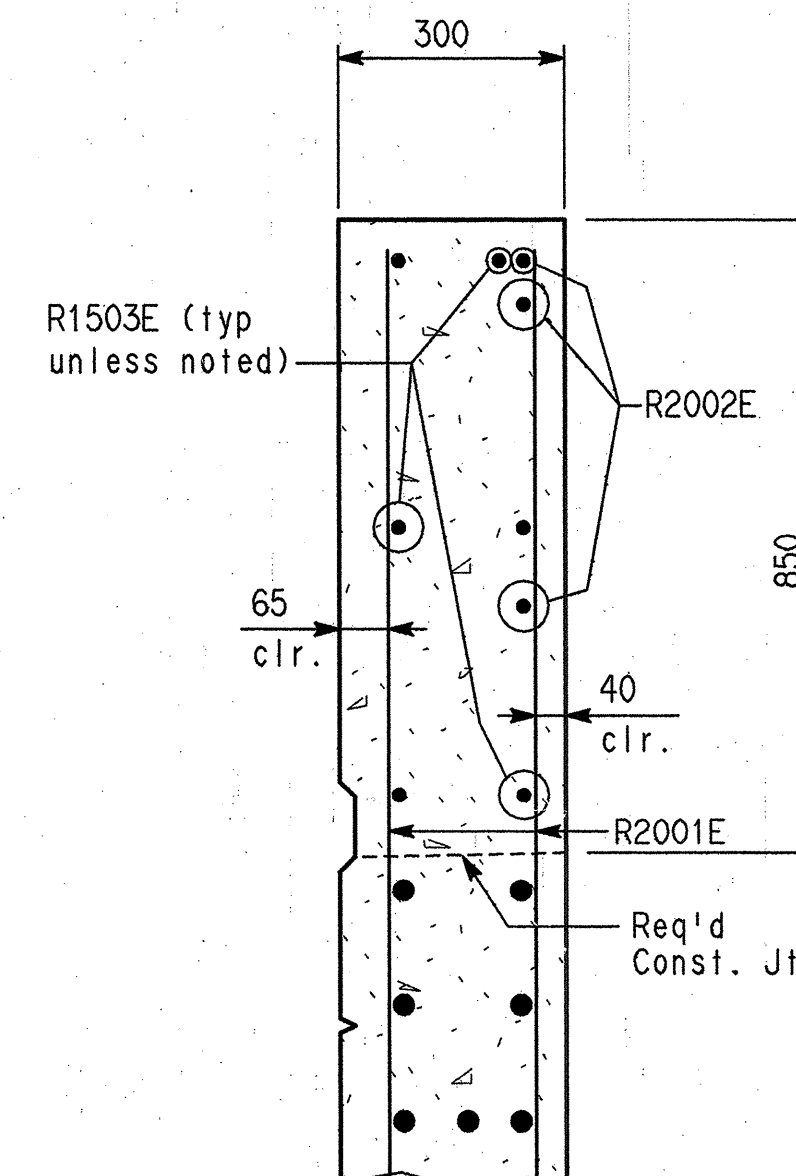
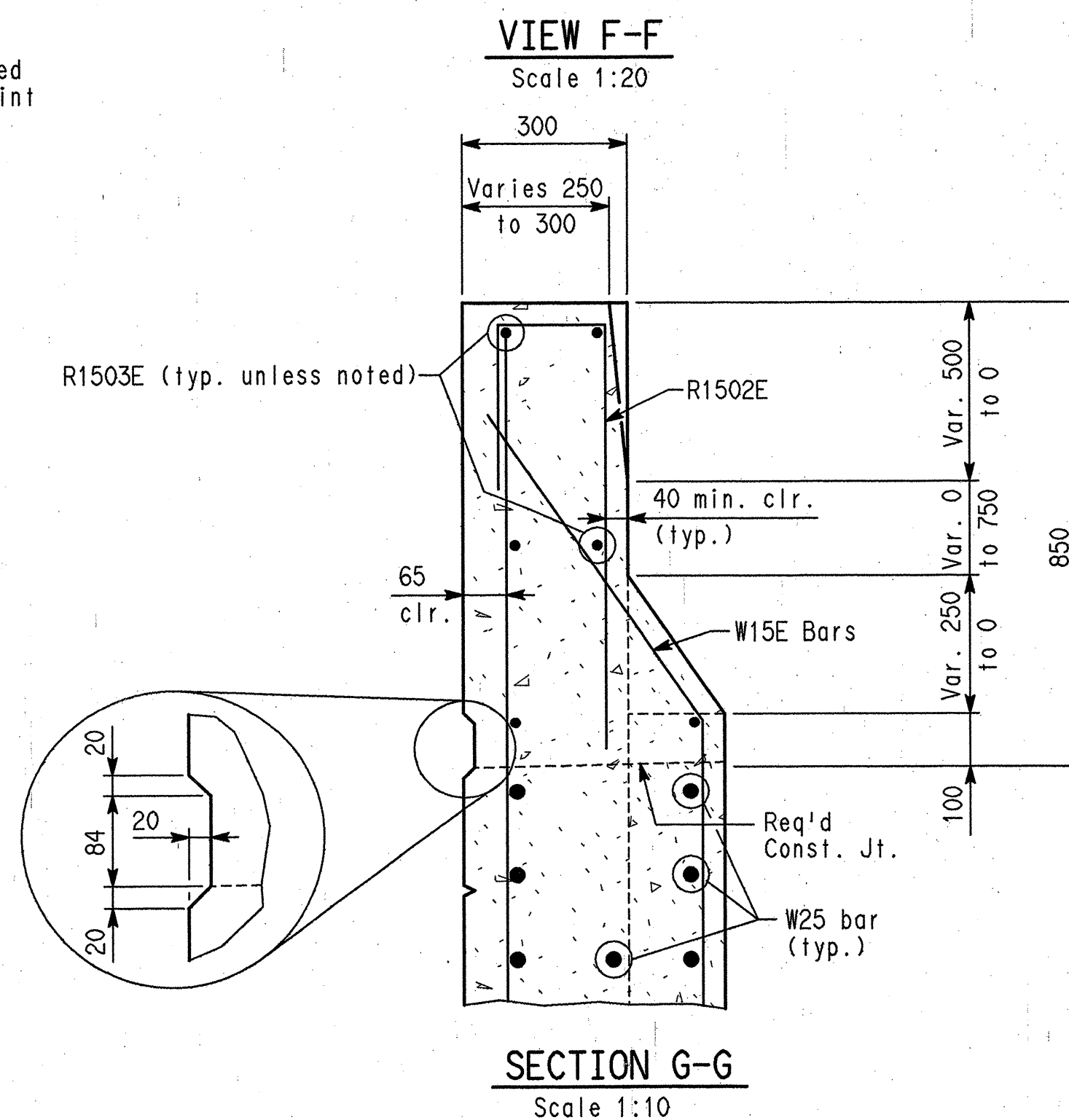
EASTBOUND BRIDGE
SHEET 1 OF 2
BENT 1
(PLAN AND ELEVATION)
US. HWY. 412
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARKANSAS

DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted
DESIGNED BY: DS DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 30244 33244

MICROFILMED
MAY 12 1997



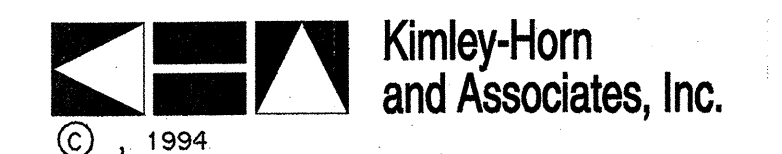
Arkansas Temporary Permit Number 96-45,
Issued 12-15-96.
Signature of Holder *Steph L. Hickey* 2/1/97



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	Fed Road Dist. No.	State	Fed. and Proj No.	Sheet No.	Total Sheets
7-14-97	7-23-97				Ark.		21	118
				Job No.		040236		
				B6686		BENT 1	38245	39245

MARK	NO. REQ'D	LENGTH	P.D.	BENDING DIAGRAMS
B1501	42	12240	Str	(Dimensions are out to out on bars)
B1502	6	11600	Str	1590
B1503	17	2600	70	545 (typ.)
B1504	4	3800	Str	300
B1505	4	1400	Str	2160
B1506	21	2720	70	1530
B1507	4	3060	Str	300
B1508	4	2820	Str	B1506
B2001	40	2580	120	545 (typ.)
B2002	76	2900	Str	1590
B2003	40	1740	120	300
B2004	4	2440	Str	550
B2005	32	1900	120	320
B2501	80	1880	150	230
B2502	88	2700	Str	540
B2503	5	11 600	Str	760
W1501	24	1400	Str	540
W1502E	12	4120	Str	800
W1503E	12	3740	70	800
W1504E-	One of	Var. 4280	Str	1540
W1506E	Each	to 3760	Str	400
W1507E-	One of	Var. 4080	Str	360
W1509E	Each	to 3580	Str	10
W1510E-	One of	Var. 3940	70	3380 (W1503E)
W1512E	Each	to 3420	70	Var. 3580 to 3060 (W1510E - W1512E)
W1513E-	One of	Var. 3740	70	Var. 3380 to 2860 (W1513E - W1515E)
W1515E	Each	to 3220	70	Var. 2880 to 1020 (W1532E - W1539E)
W1516E-	One of	Var. 3580	Str	Var. 2720 to 1000 (W1540E - W1547E)
W1523E	Each	to 1720	Str	W1503E, W1510E-W1515E, W1532E-W1547E
W1524E-	One of	Var. 3420	Str	150
W1531E	Each	to 1700	Str	10
W1532E-	One of	Var. 3240	70	300
W1539E	Each	to 1380	70	10
W1540E-	One of	Var. 3080	70	775
W1547E	Each	to 1360	70	300
W2501	20	5400	Str	1
W2502-	Two of	Var. 1540	Str	800
W2516	Each	to 4540	Str	1
W2517-	Two of	Var. 1540	Str	4800
W2530	Each	to 4540	Str	W2531
W2531	4	5600	150	
R1501E	26	1150	70	
R1502E	8	1180	70	
R1503E	12	5400	Str	
R2001E	16	1660	Str	
R2002E	6	1560	Str	

Details shown are for Right Wing, Left Wing is similar.



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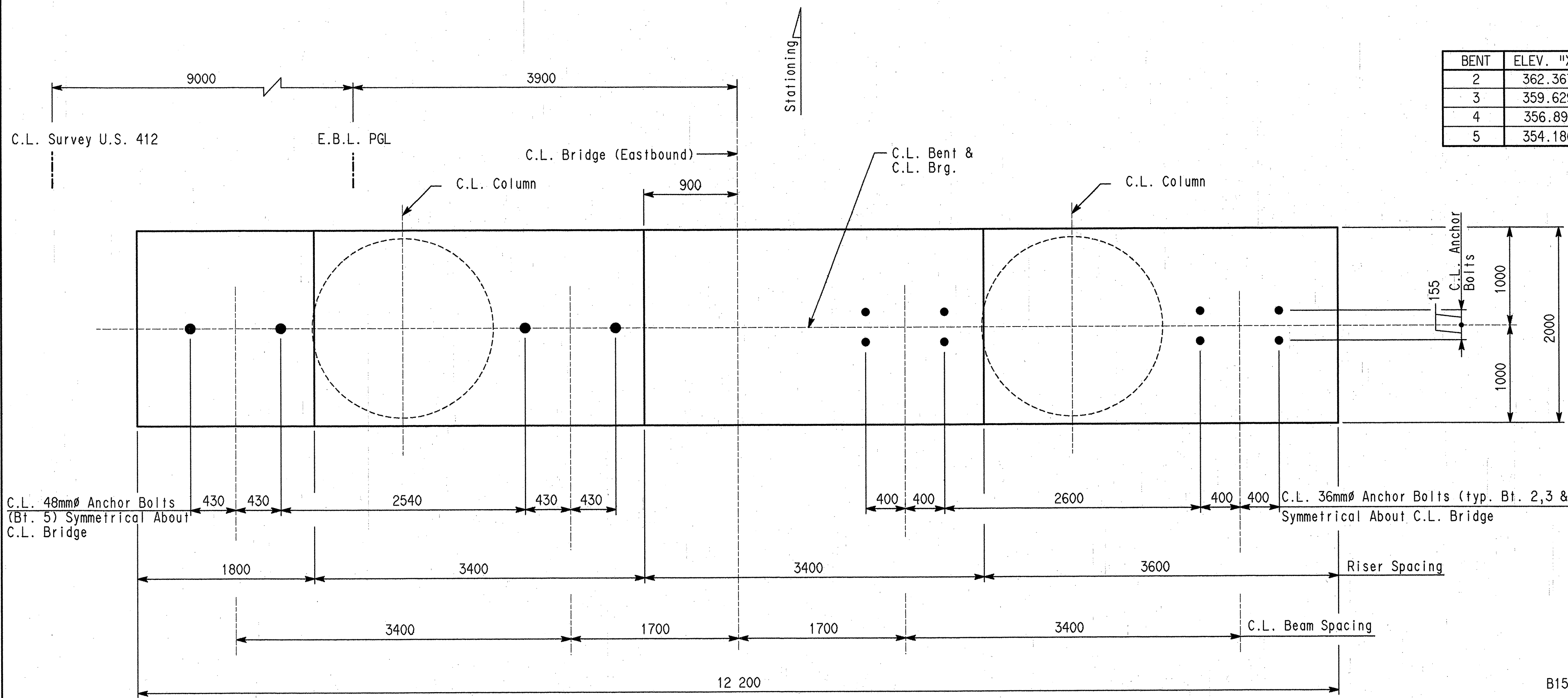
EASTBOUND BRIDGE
SHEET 2 OF 2
BENT 1
(WING DETAIL)
US. HWY. 412
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARKANSAS

DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted
DESIGNED BY: DS DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 38245 39245

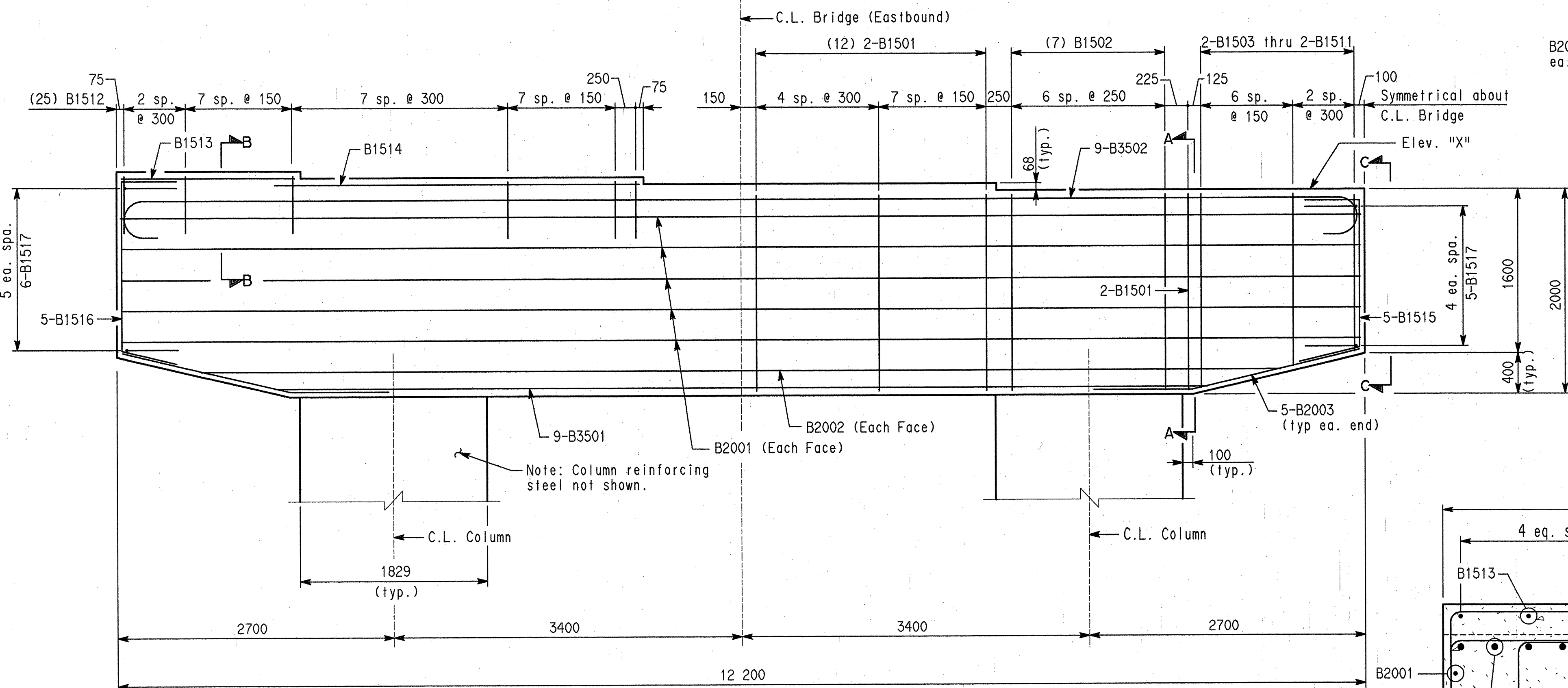
All dimensions are in millimeters (mm) unless otherwise noted.

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				B6686		BENTS 2 THRU 5	30246 39246	

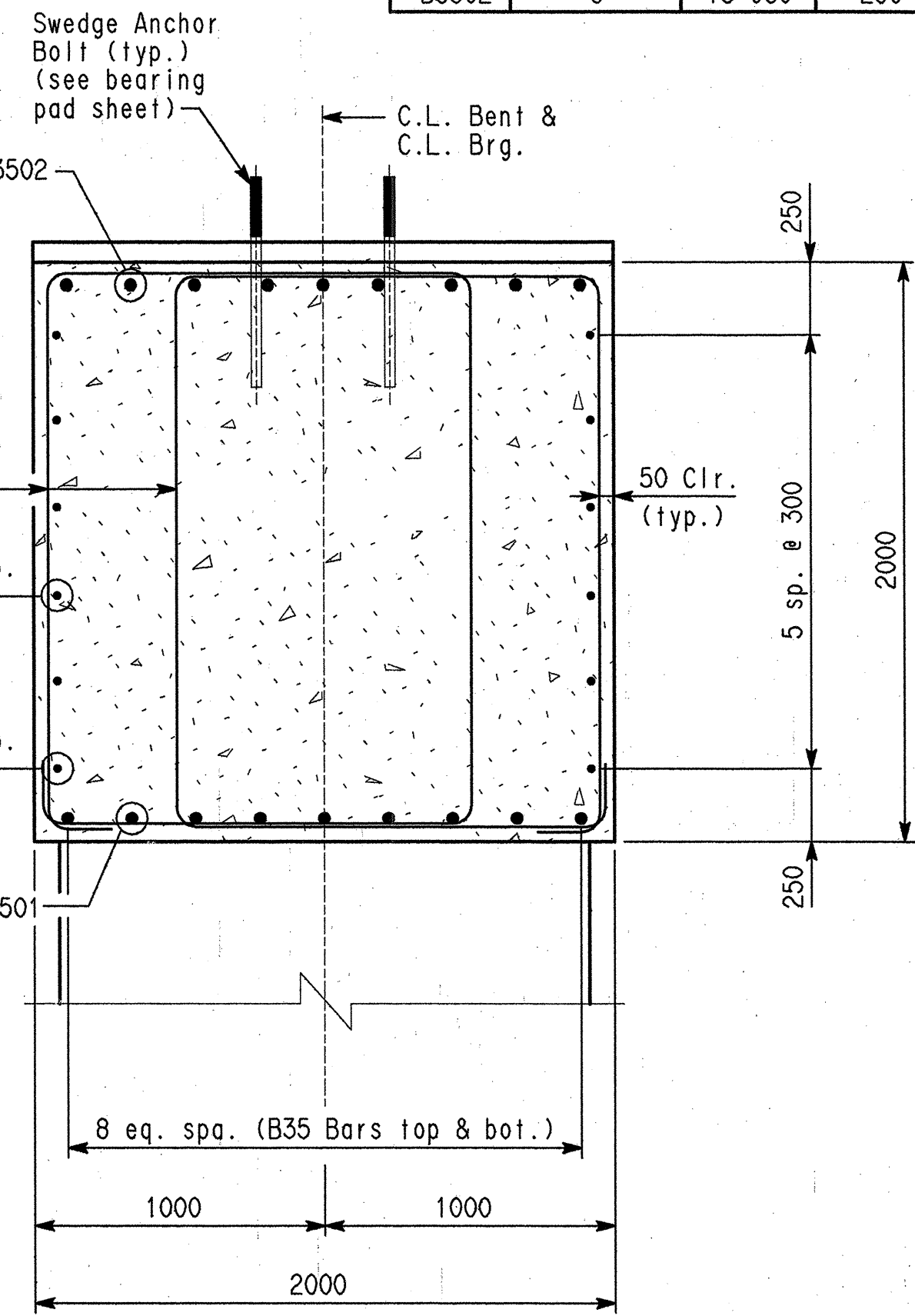
BENT	ELEV. "X"
2	362.367
3	359.629
4	356.891
5	354.186



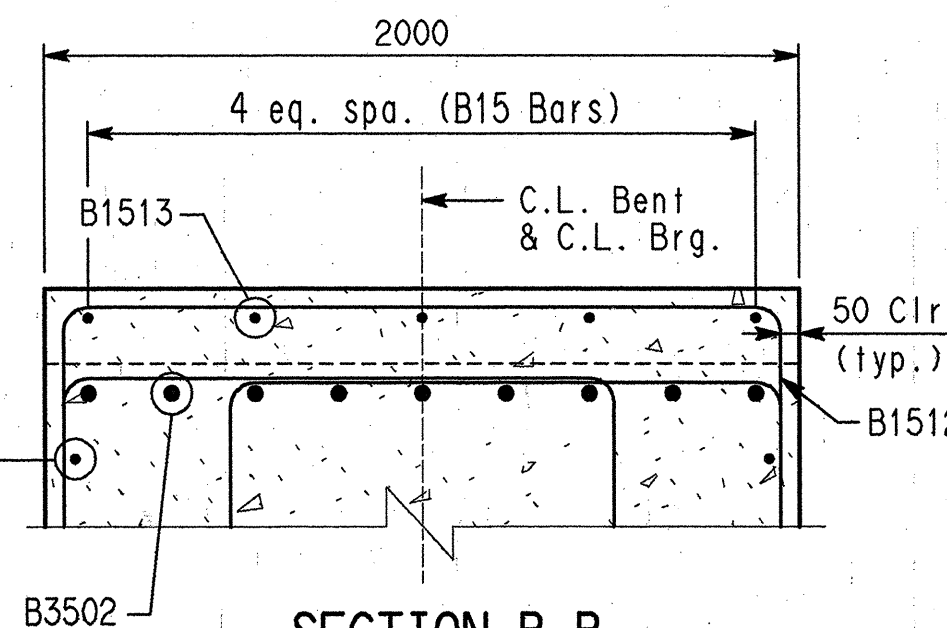
PLAN
Scale 1:30



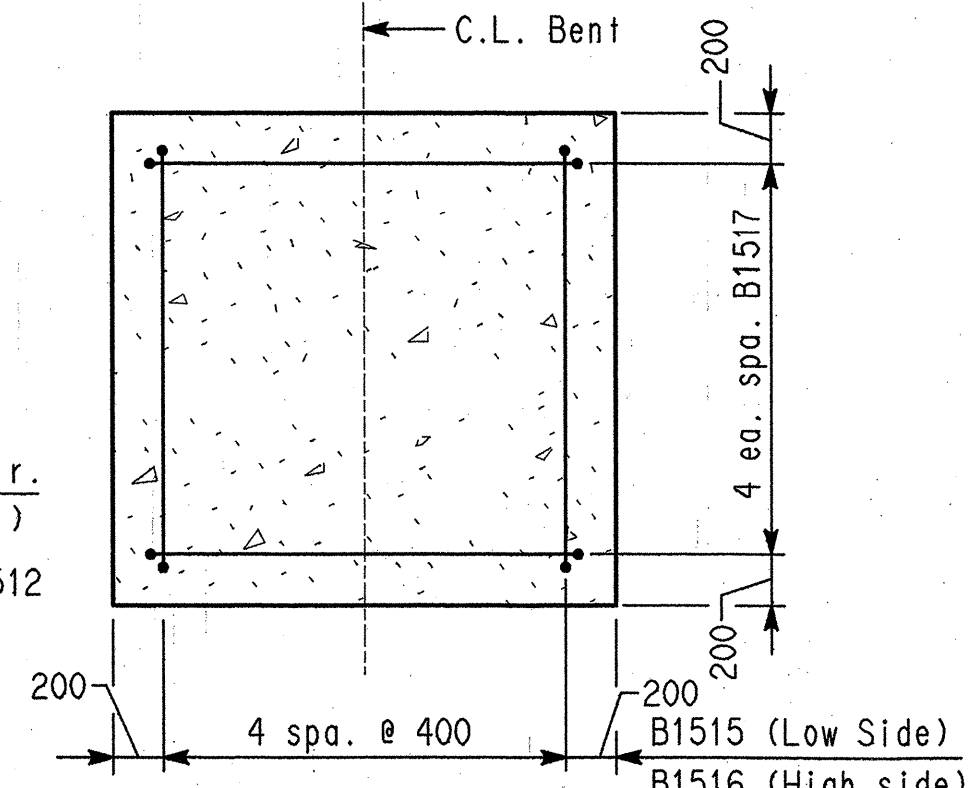
ELEVATION
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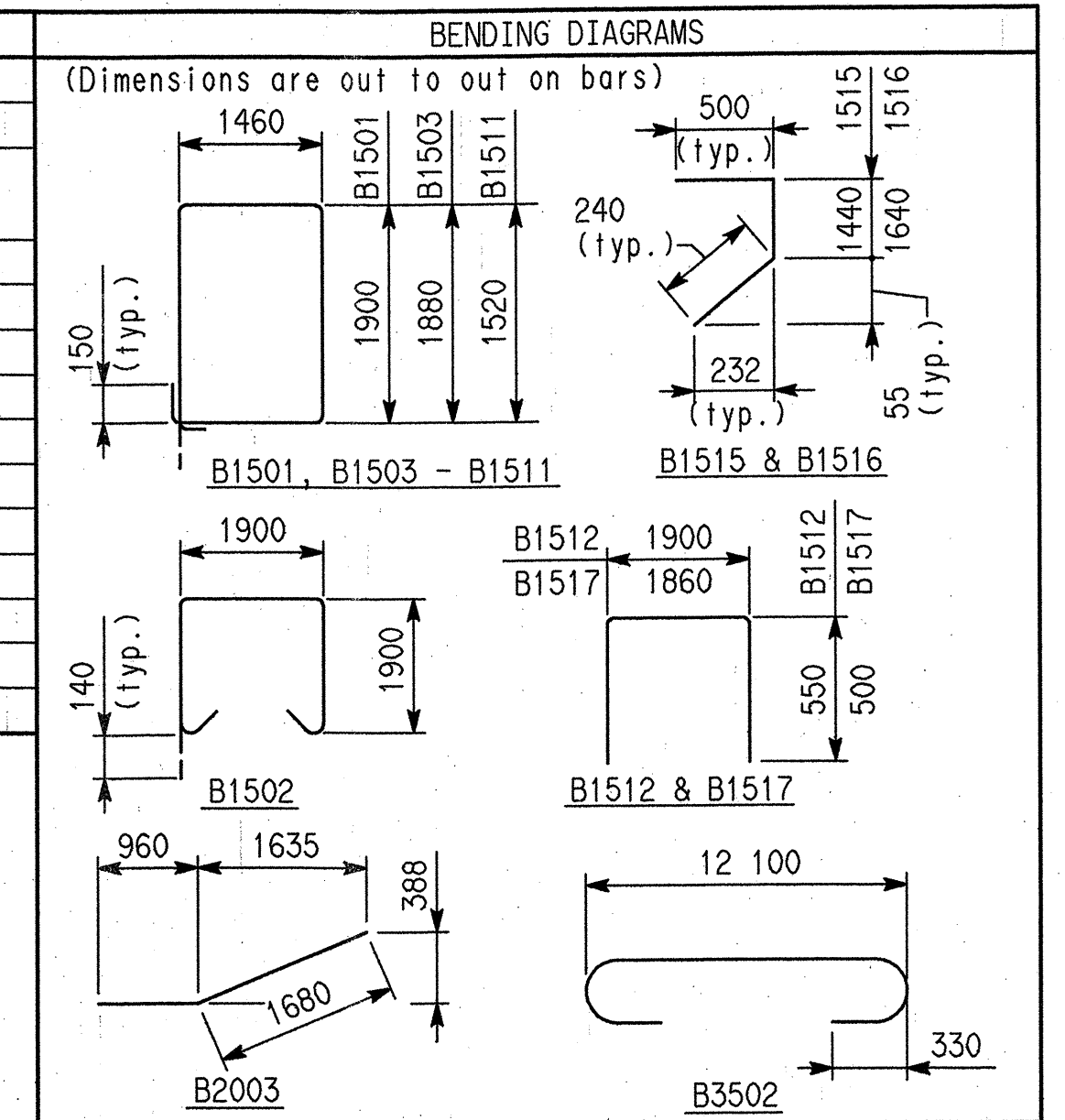
SECTION A-A
Scale 1:20



SECTION B-B
Scale 1:20



SECTION C-C
No Scale



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Issued 12-15-96.
Signature of Holder *SLH* 12/15/96

NOTES
Stations and elevations are in meters (m).
All other dimensions are in millimeters (mm)
unless otherwise noted.

All concrete shall be Class "S" with a minimum
28 day compressive strength $f'_c = 24$ MPa.
Concrete shall be poured in the dry and all exposed
corners to be chamfered 20 unless otherwise noted.

All reinforcing steel shall conform to AASHTO
M 31M or M 53M, Gr. 400 (yield strength = 400 MPa)

If anchor bolts are drilled into the cap, top
reinforcing shall be properly placed to avoid damage

For additional information see layout.

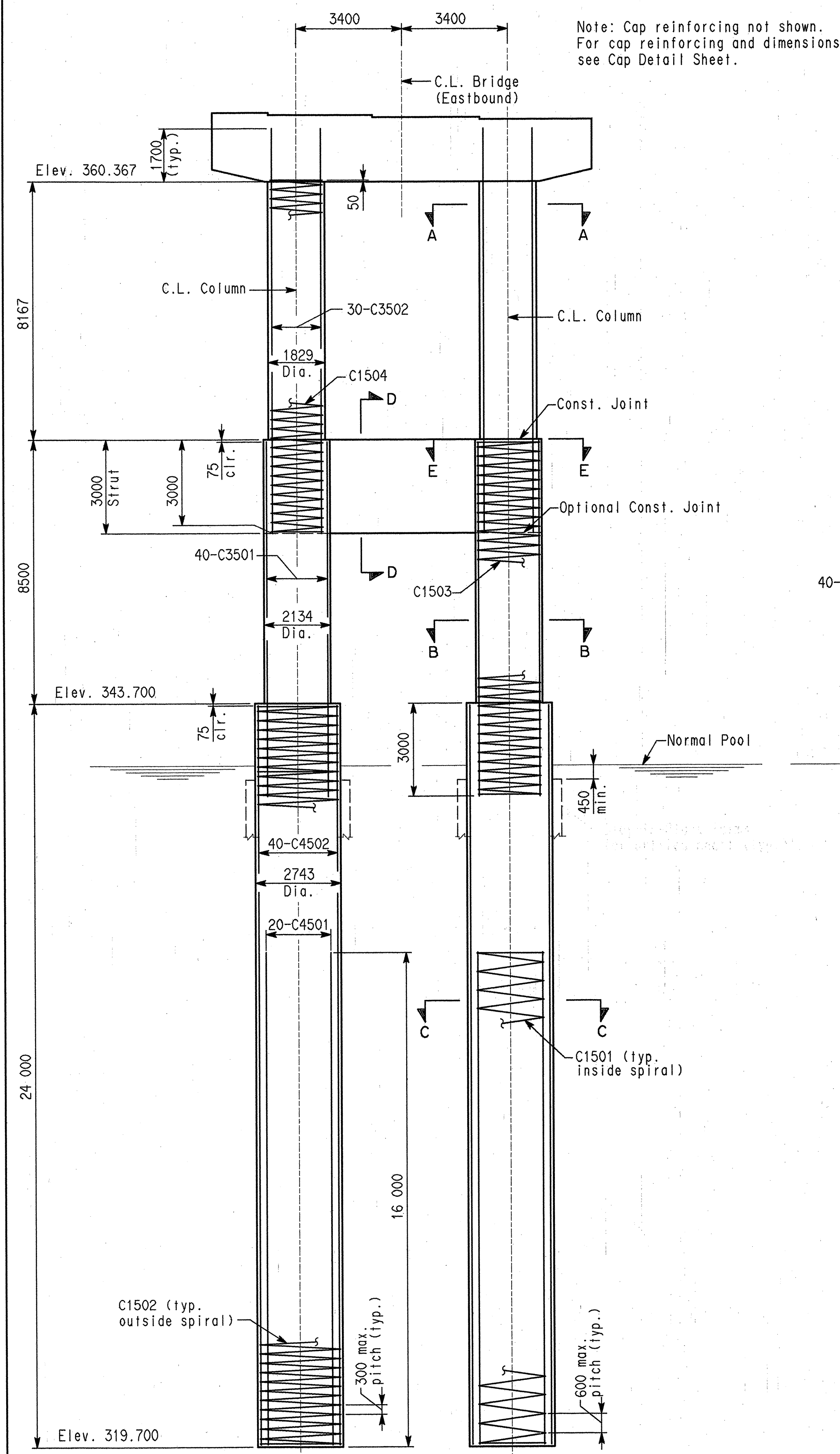
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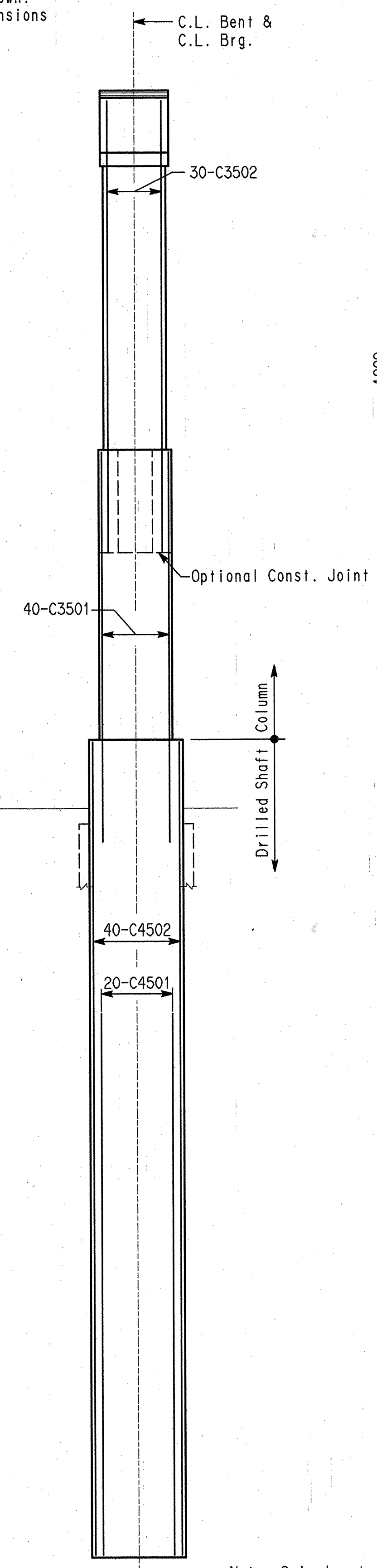
**EASTBOUND BRIDGE
SHEET 1 OF 1
BENT 2 THRU 5
CAP DETAILS**
US. HWY. 412
**ARKANSAS STATE HIGHWAY
COMMISSION**
LITTLE ROCK, ARKANSAS

DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted
DESIGNED BY: SLH DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 30246 39246

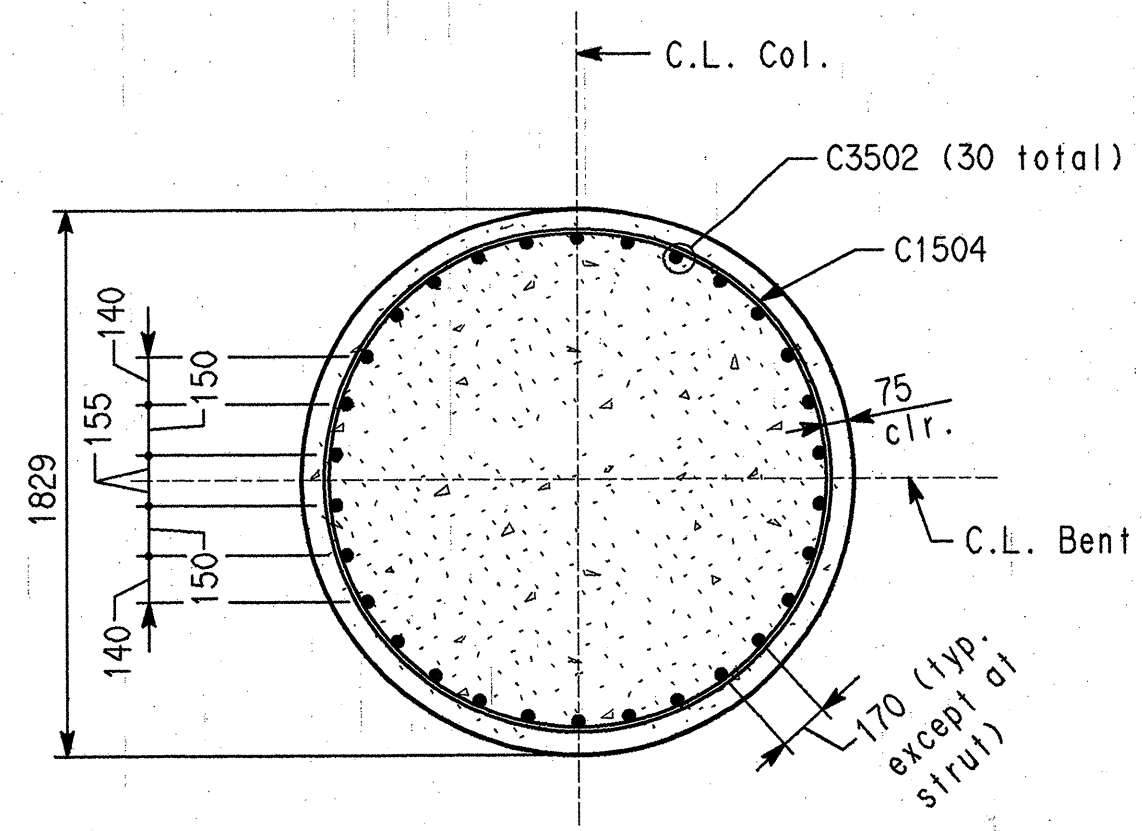
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				B6686		BENT 2	39247	39247



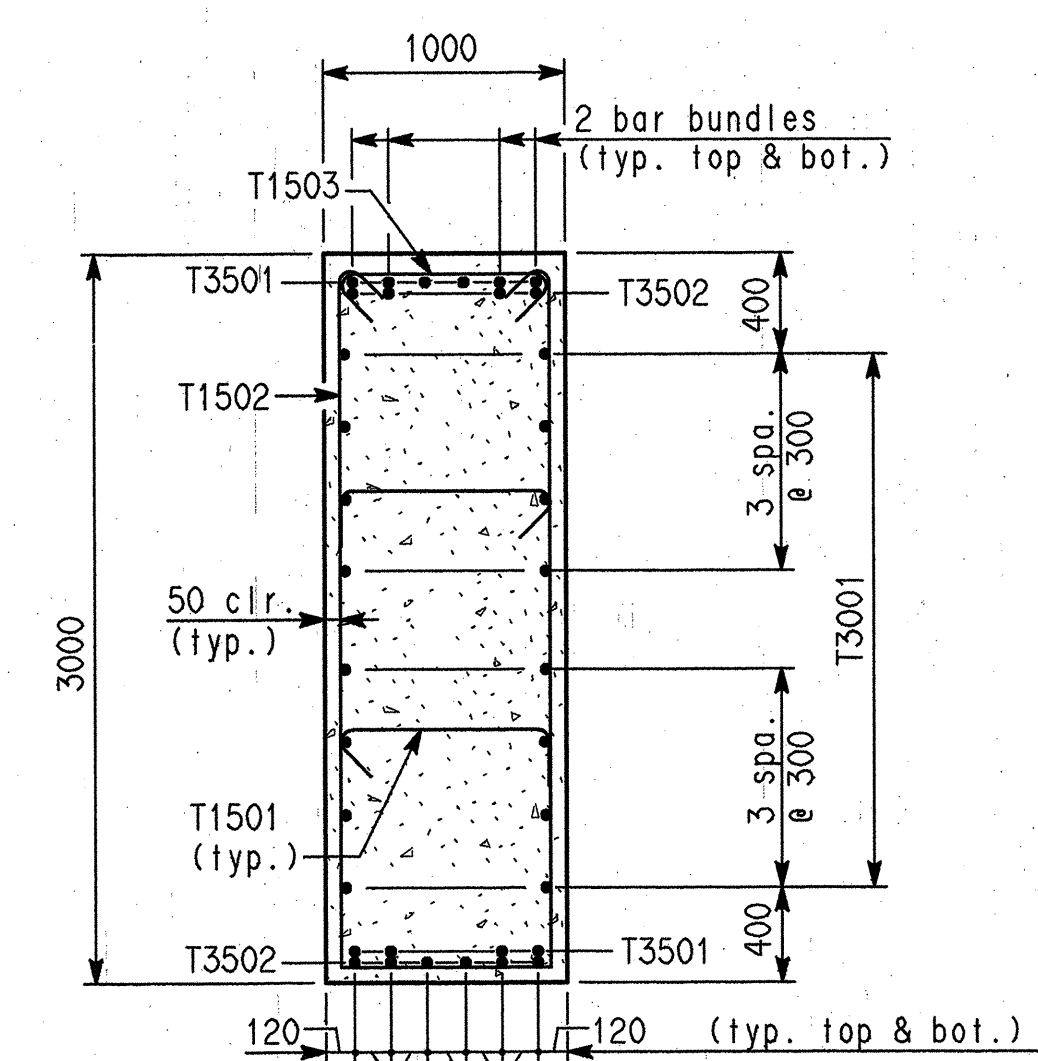
ELEVATION - LOOKING AHEAD
Scale 1:100



END ELEVATION
Scale 1:100

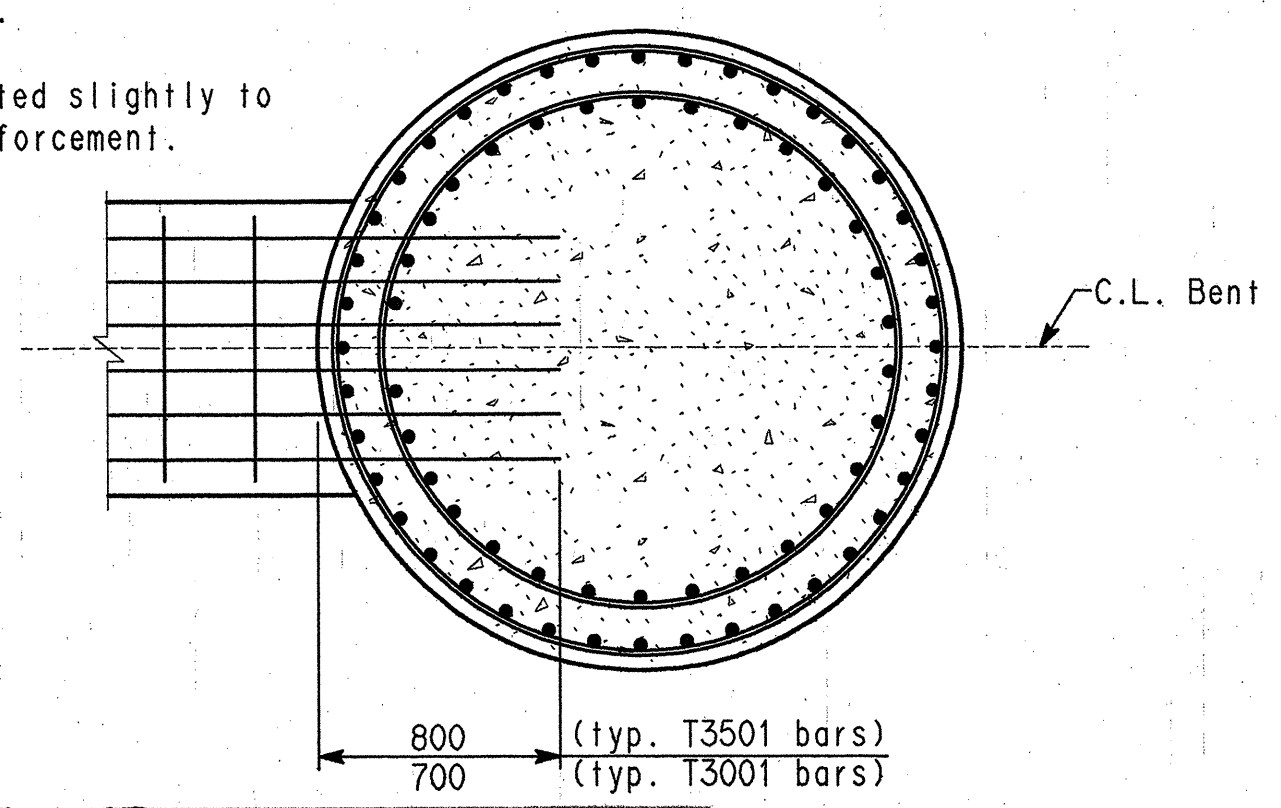


SECTION A-A
Scale 1:25

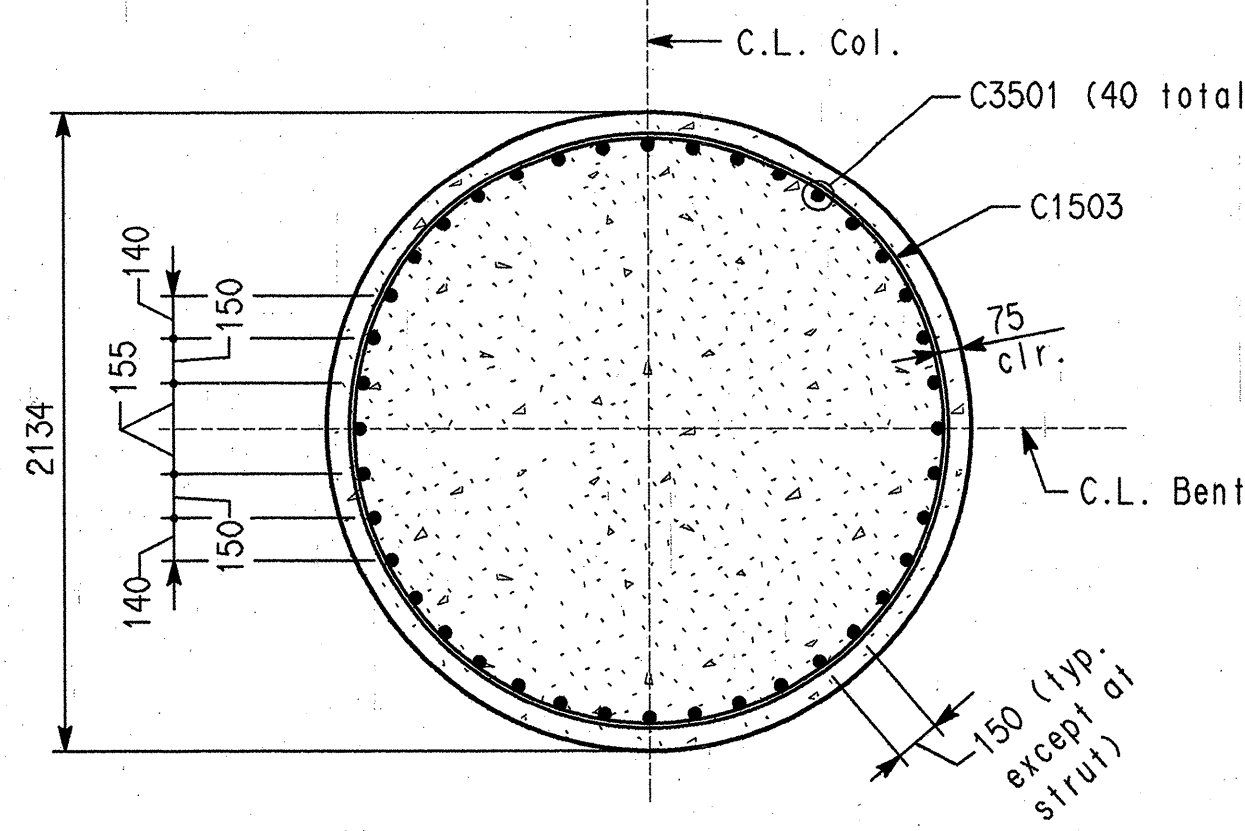


SECTION D-D
No Scale

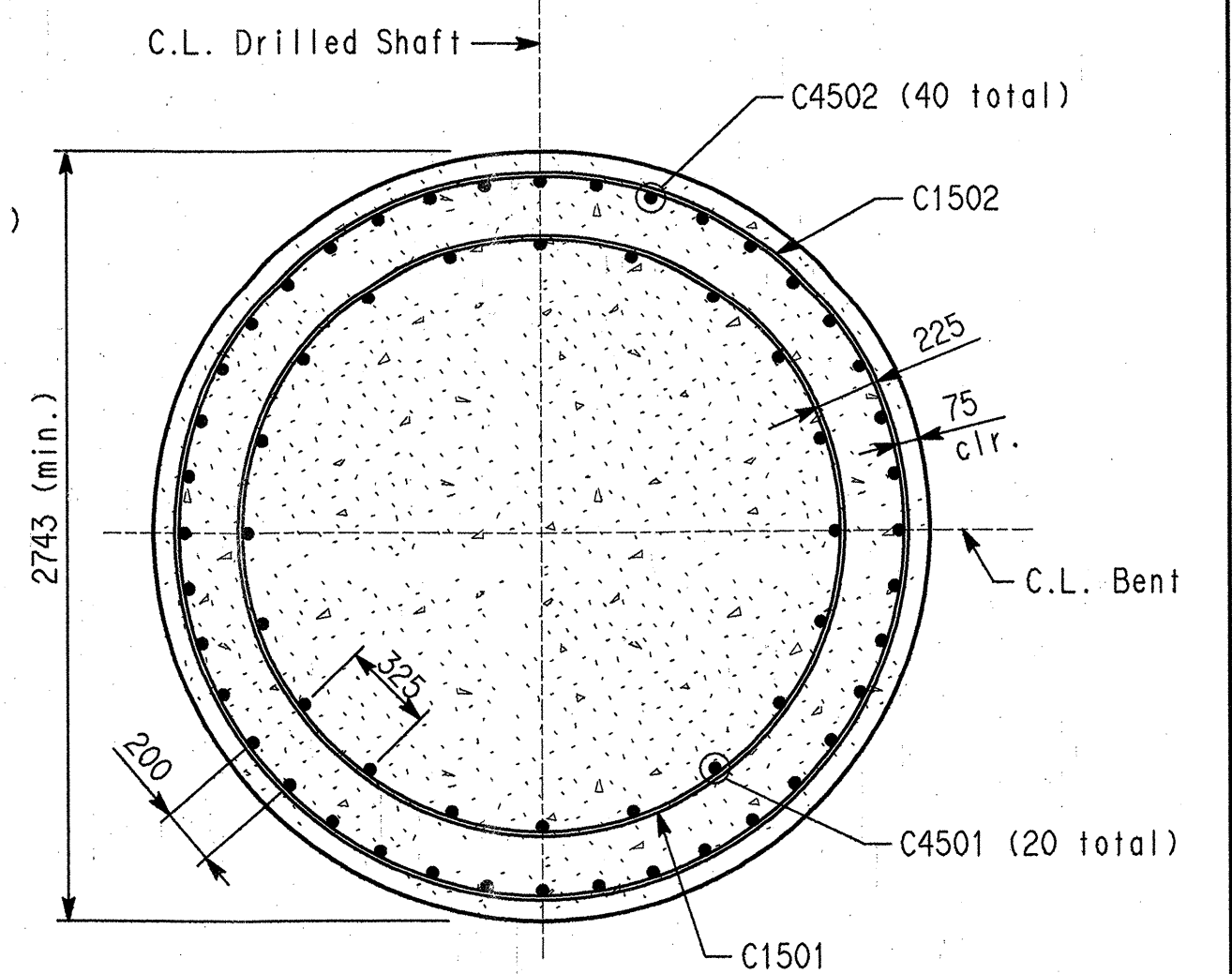
Note: Alternate the 135° hook on each side of strut for the T1501 bars in each row.
T3001 maybe shifted slightly to pass column reinforcement.



SECTION E-E
No Scale

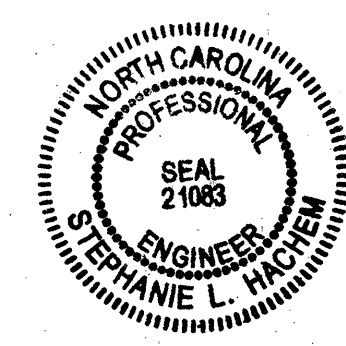


SECTION B-B
Scale 1:25

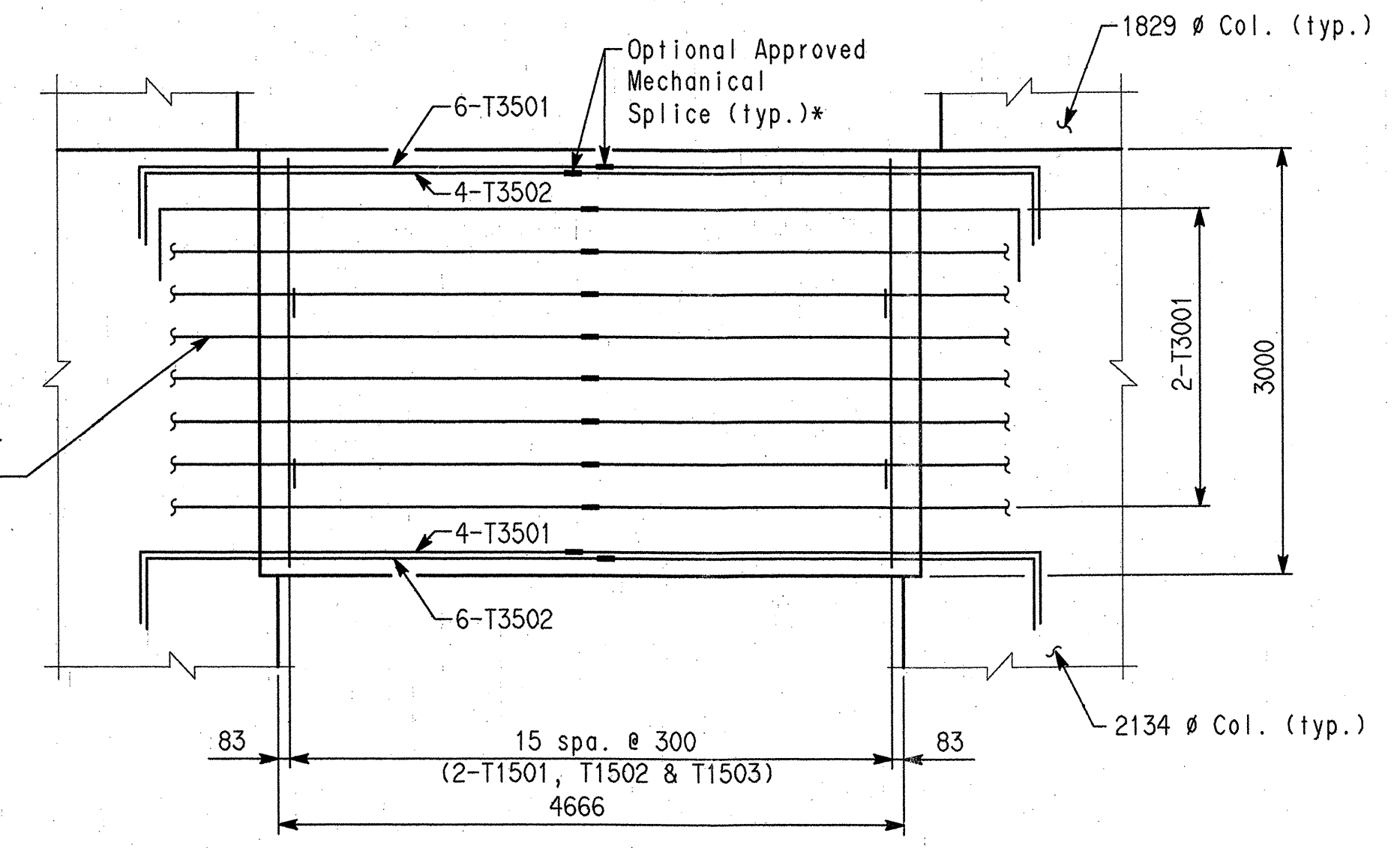


SECTION C-C
Scale 1:25

T3001 Bars shall be twisted to clear other T3001 Bars.



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Signature of Holder *[Signature]* 4/2/97



STRUT ELEVATION
No Scale

*No extra payment will be made for optional mechanical splices.

NOTES
Stations and elevations are in meters (m). All other dimensions are in millimeters (mm) unless otherwise noted.
Bent concrete shall be Class "S" with a minimum 28 day compressive strength $f'_c = 24$ MPa. Concrete shall be poured in the dry and all exposed corners to be chamfered 20 unless otherwise noted.
Drilled Shafts concrete shall be Class "S" (modified) with a minimum 28 day compressive strength $f'_c = 28$ MPa.
All reinforcing steel shall conform to AASHTO M 31M or M 53M, Gr. 400 (yield strength = 400 MPa)
For additional information see layout.
For drilled shafts information see reinforcing bar list sheet.

All dimensions are in millimeters (mm) unless otherwise noted.

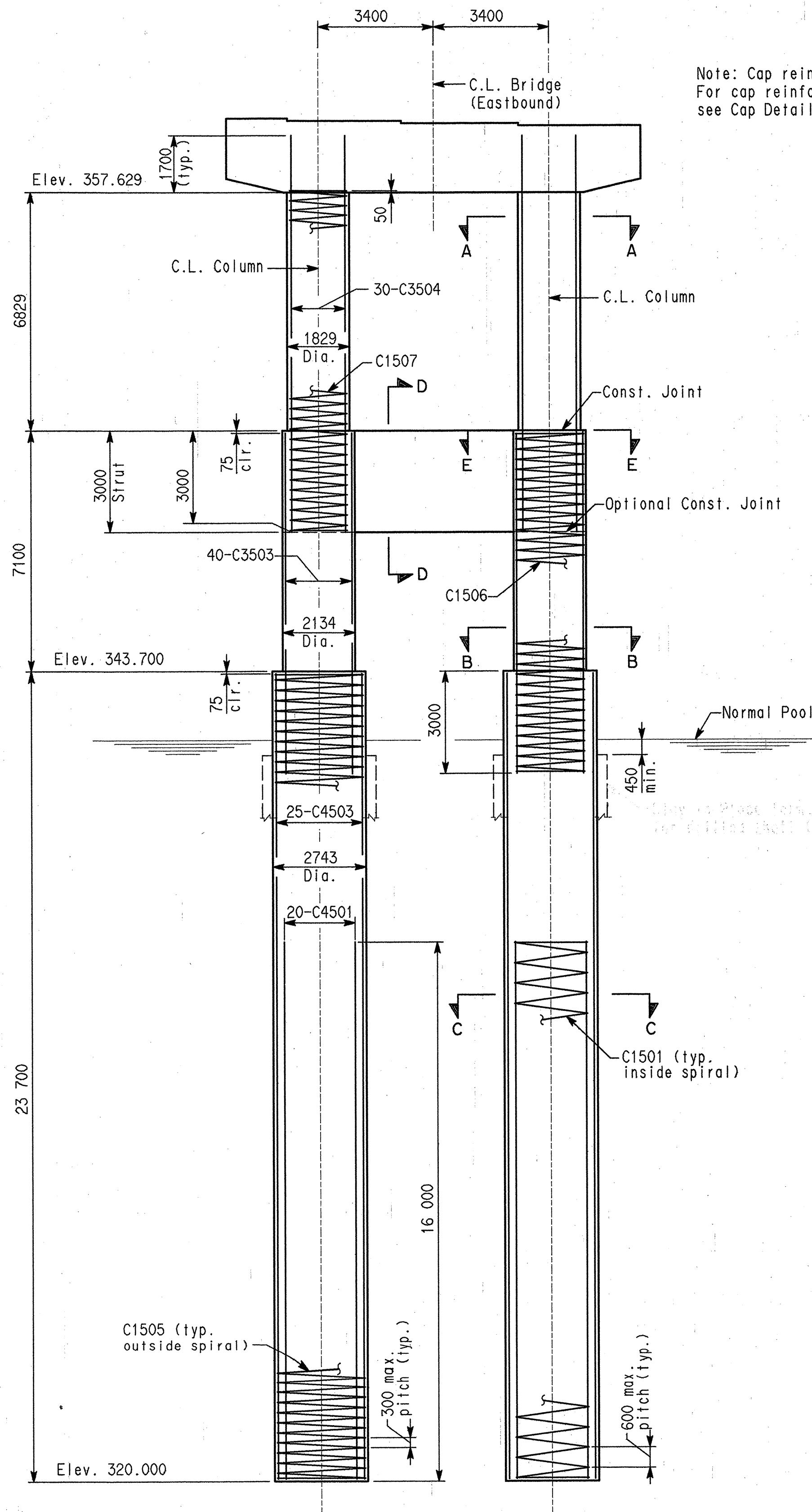


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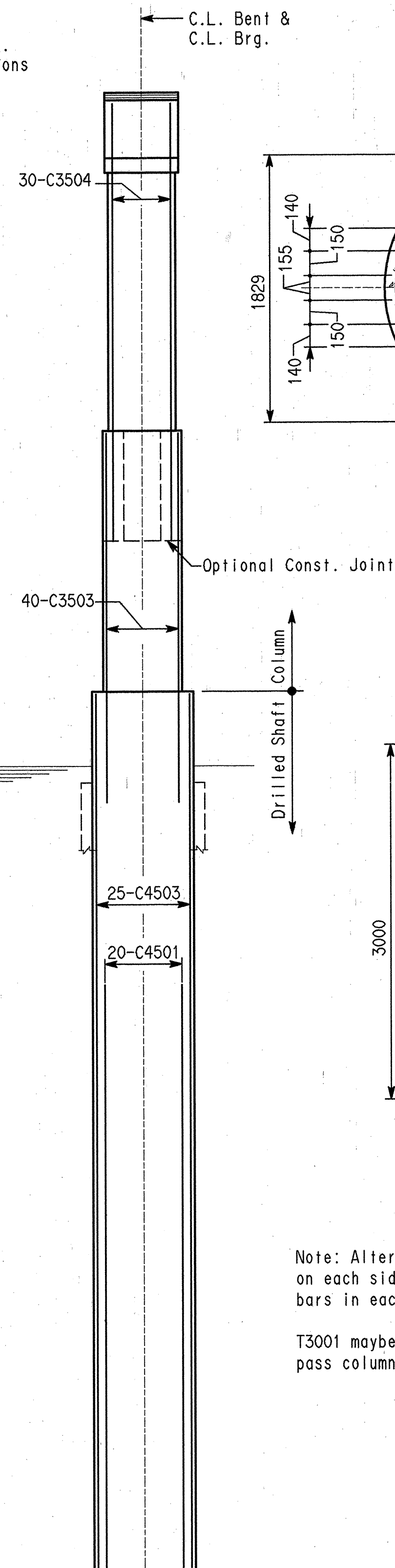
EASTBOUND BRIDGE
SHEET 1 OF 1
BENT 2
(COLUMNS)
US. HWY. 412
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARKANSAS
DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted
DESIGNED BY: SLH DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 39247

MICROFILMED
MAY 12 1997

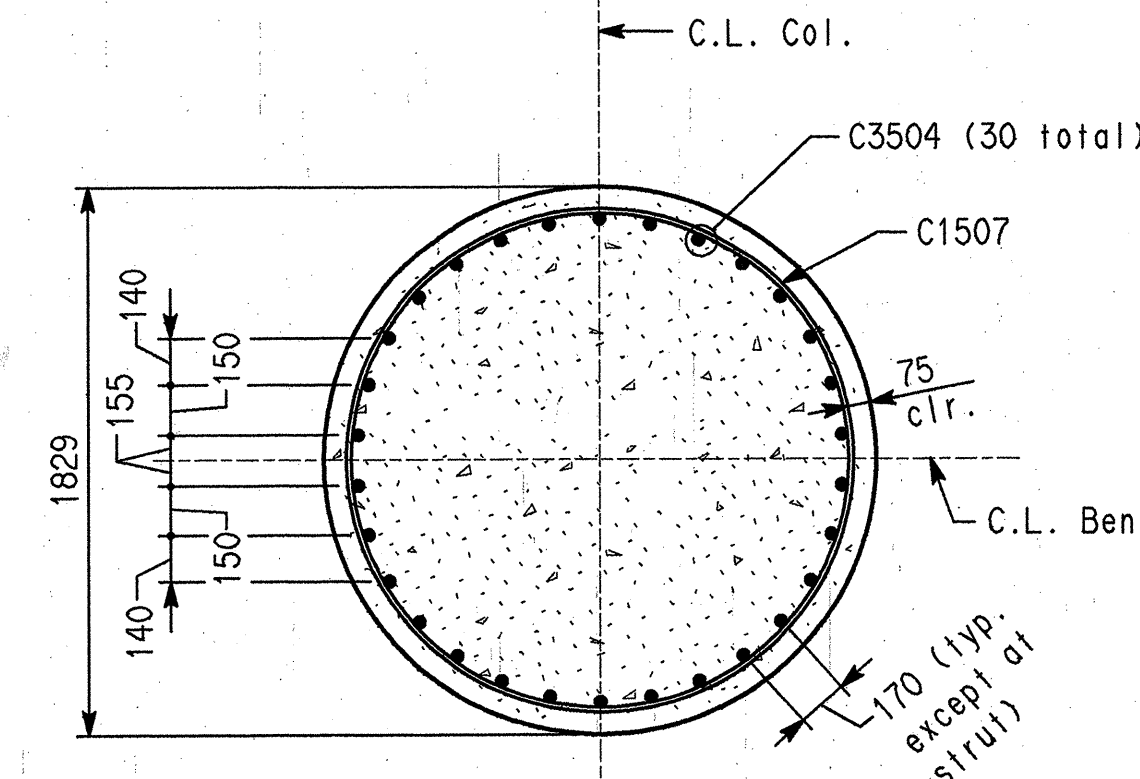
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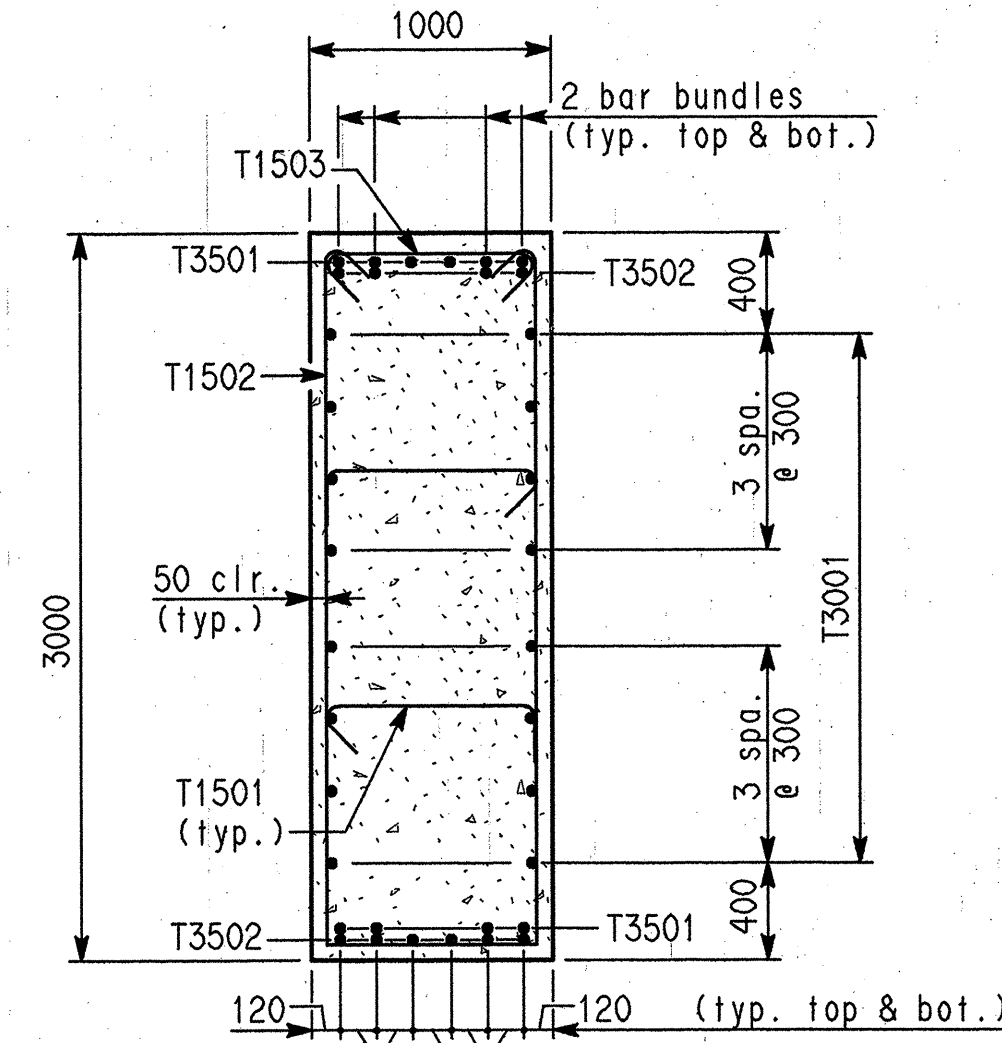
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Scale 1:100



END ELEVATION
Scale 1:100



SECTION A-A
Scale 1:25



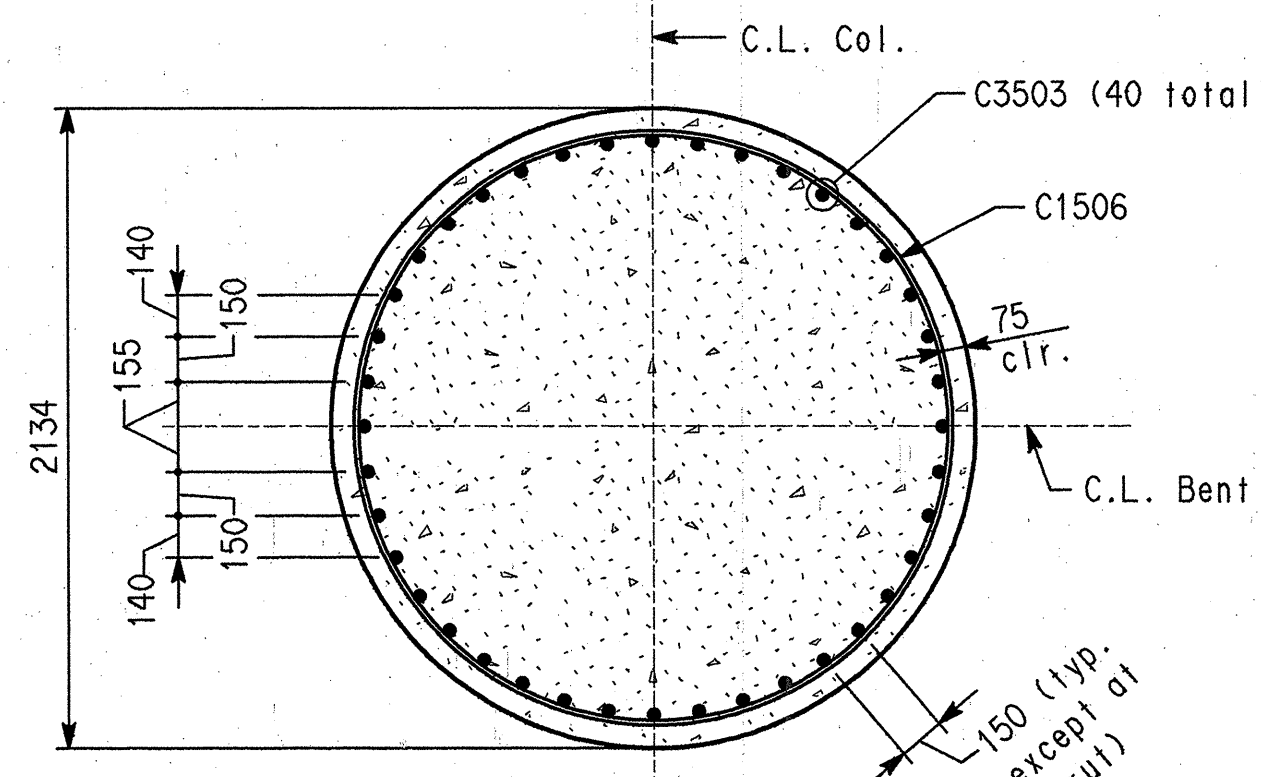
SECTION D-D
No Scale

Note: Alternate the 135° hook on each side of strut for the T1501 bars in each row.

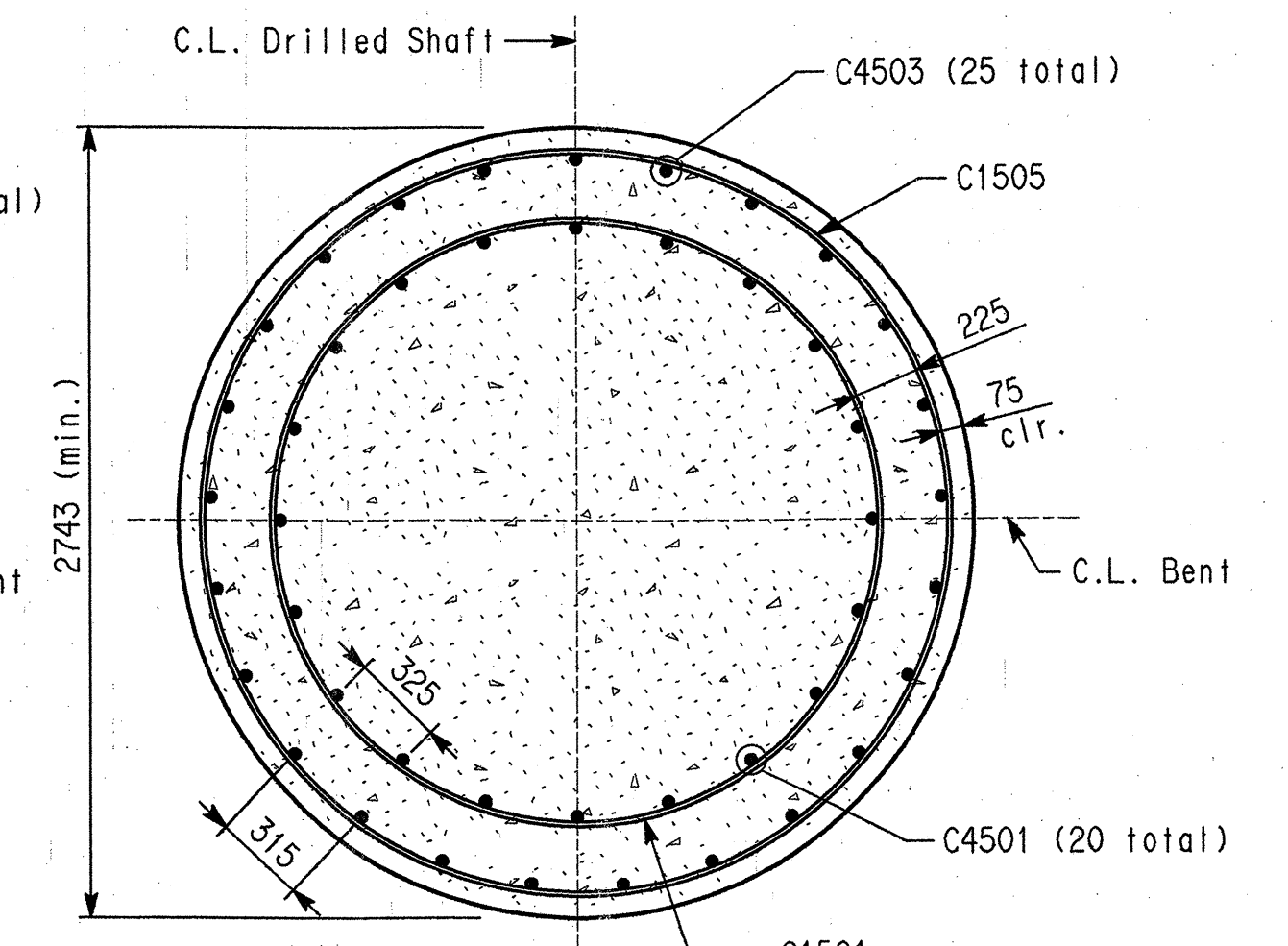
T3001 maybe shifted slightly to pass column reinforcement.

Note: Spiral not shown in the End Elevation

Rev. dwg. no. WRR 7-14-97



SECTION B-B
Scale 1:25



SECTION C-C
Scale 1:25

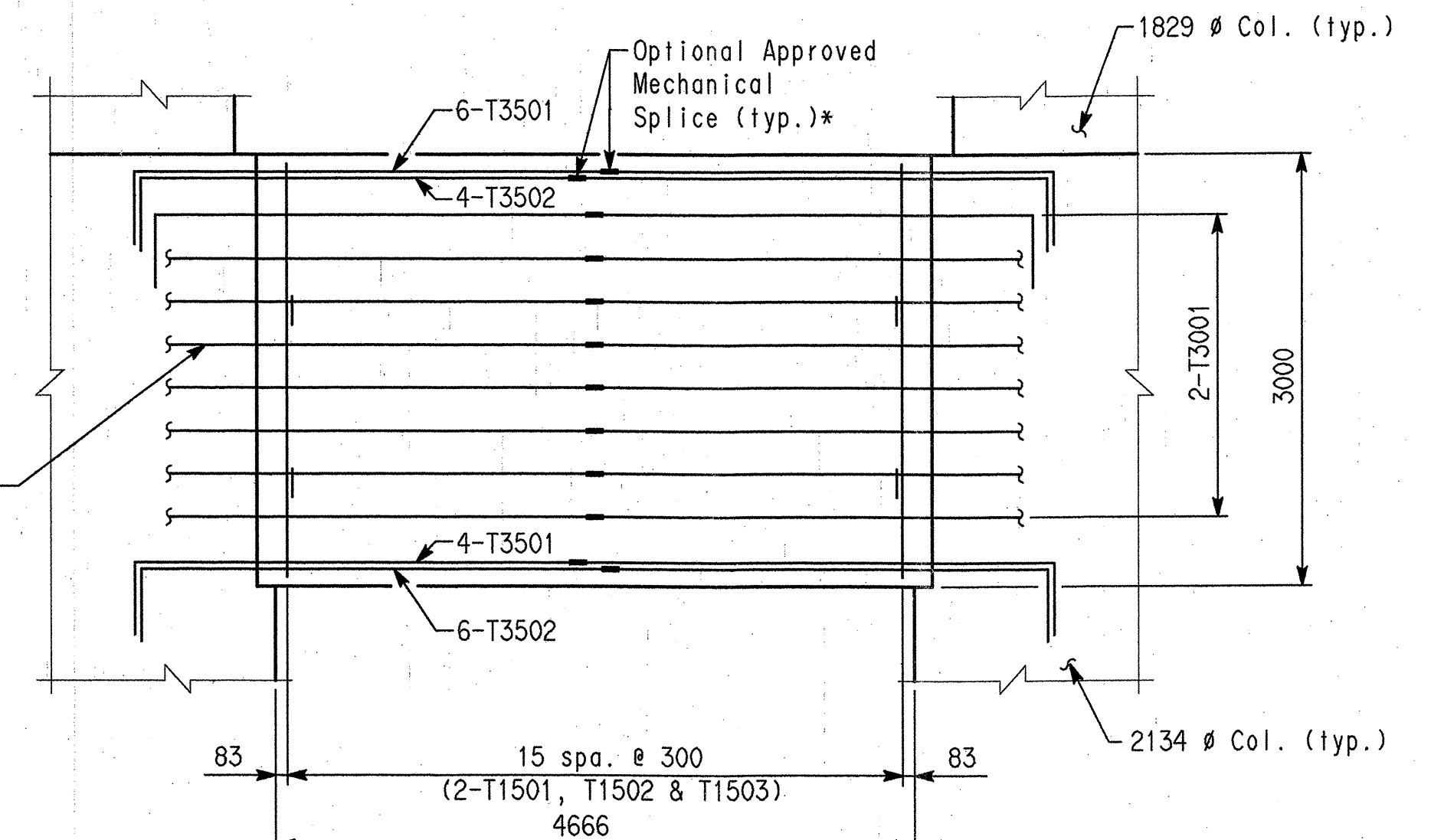


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Issued 12-15-96.
Signature of Holder *SLH* 2/2/97



NOTES
Stations and elevations are in meters (m).
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Concrete shall be poured in the dry and all exposed corners to be chamfered 20 unless otherwise noted.
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All reinforcing steel shall conform to AASHTO M 31M or M 53M, Gr. 400 (yield strength = 400 MPa)
For additional information see layout.
For drilled shafts information see reinforcing bar list sheet.

All dimensions are in millimeters (mm) unless otherwise noted.



STRUT ELEVATION
No Scale

*No extra payment will be made for optional mechanical splices.

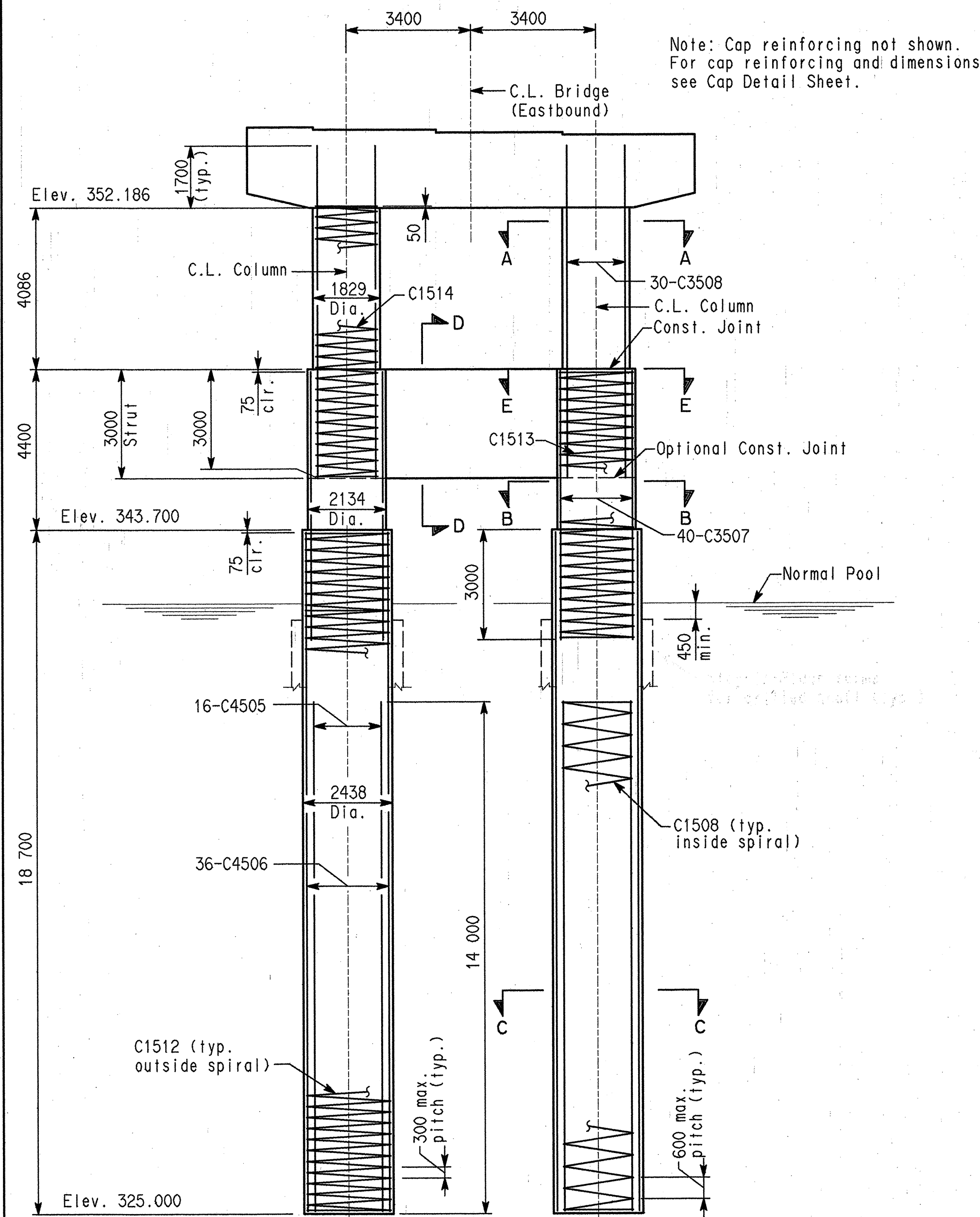


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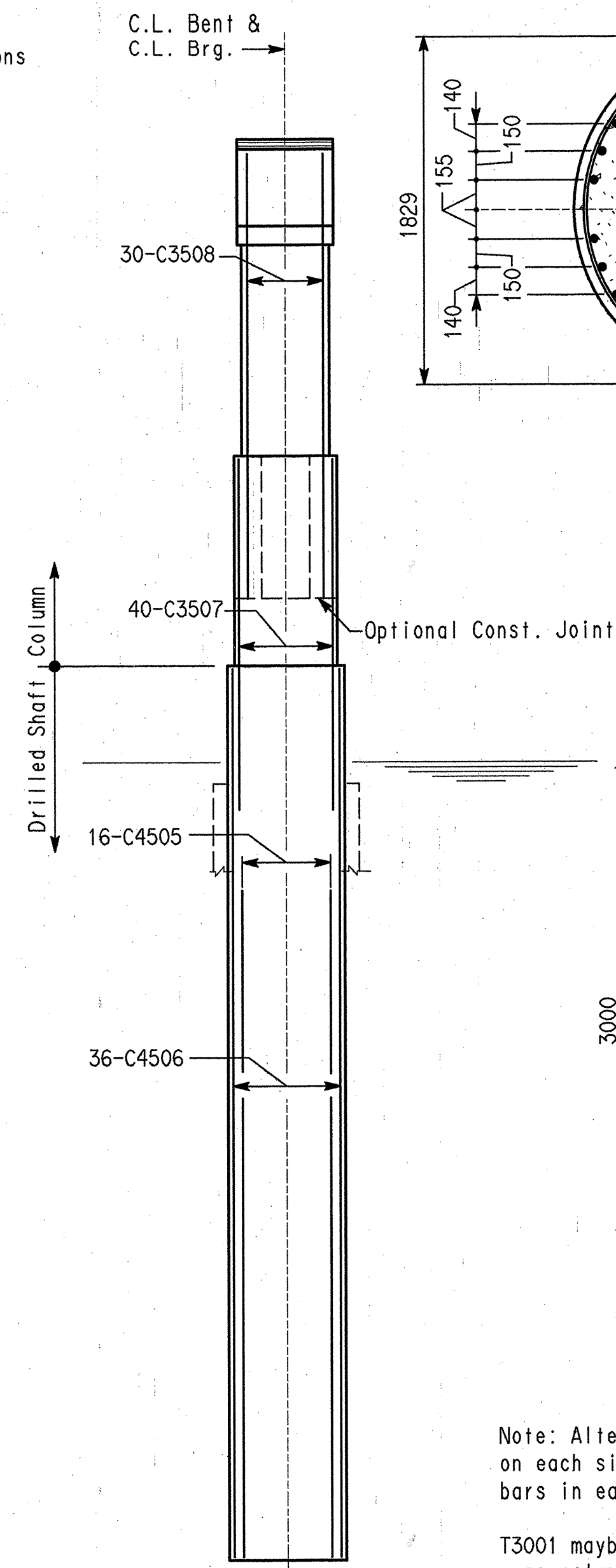
EASTBOUND BRIDGE
SHEET 1 OF 1
BENT 3
(COLUMNS)
US. HWY. 412
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARKANSAS

DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted
DESIGNED BY: SLH DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 39248

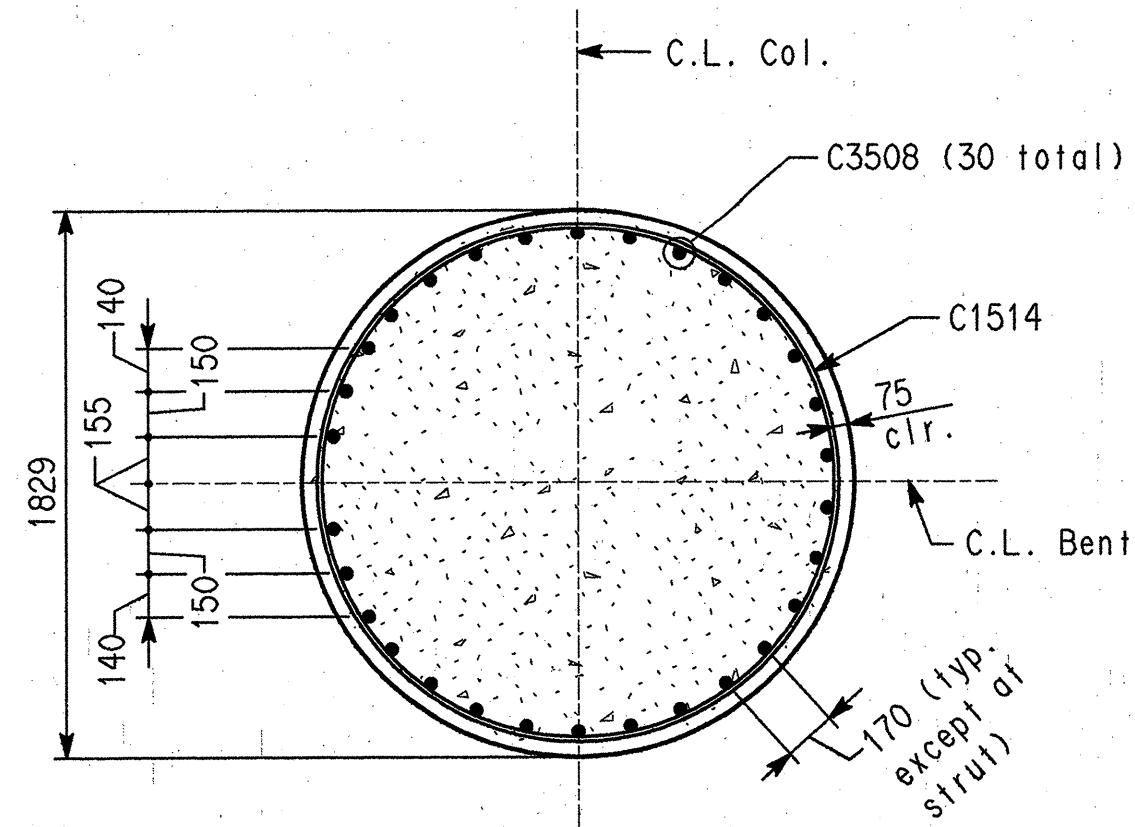
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				Job No.		040236		
				B6686		BENT 5		39250



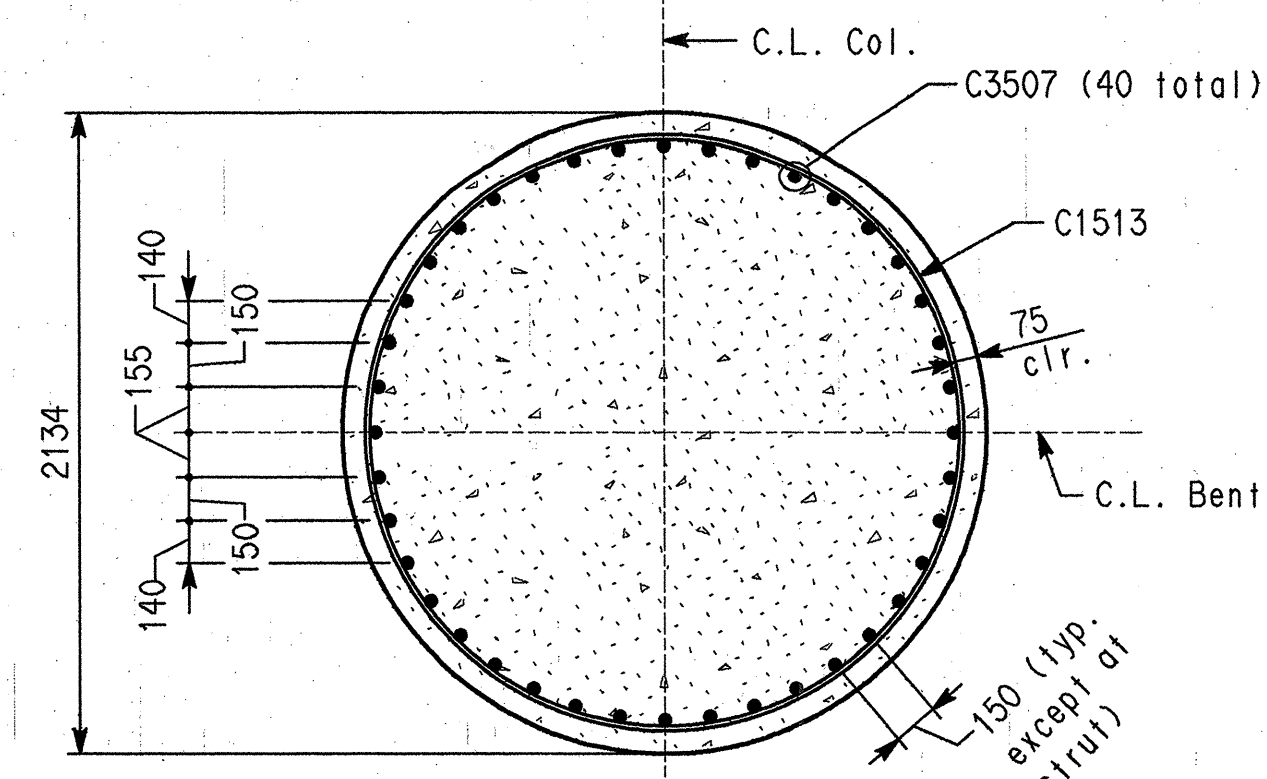
ELEVATION - LOOKING AHEAD
Scale 1:100



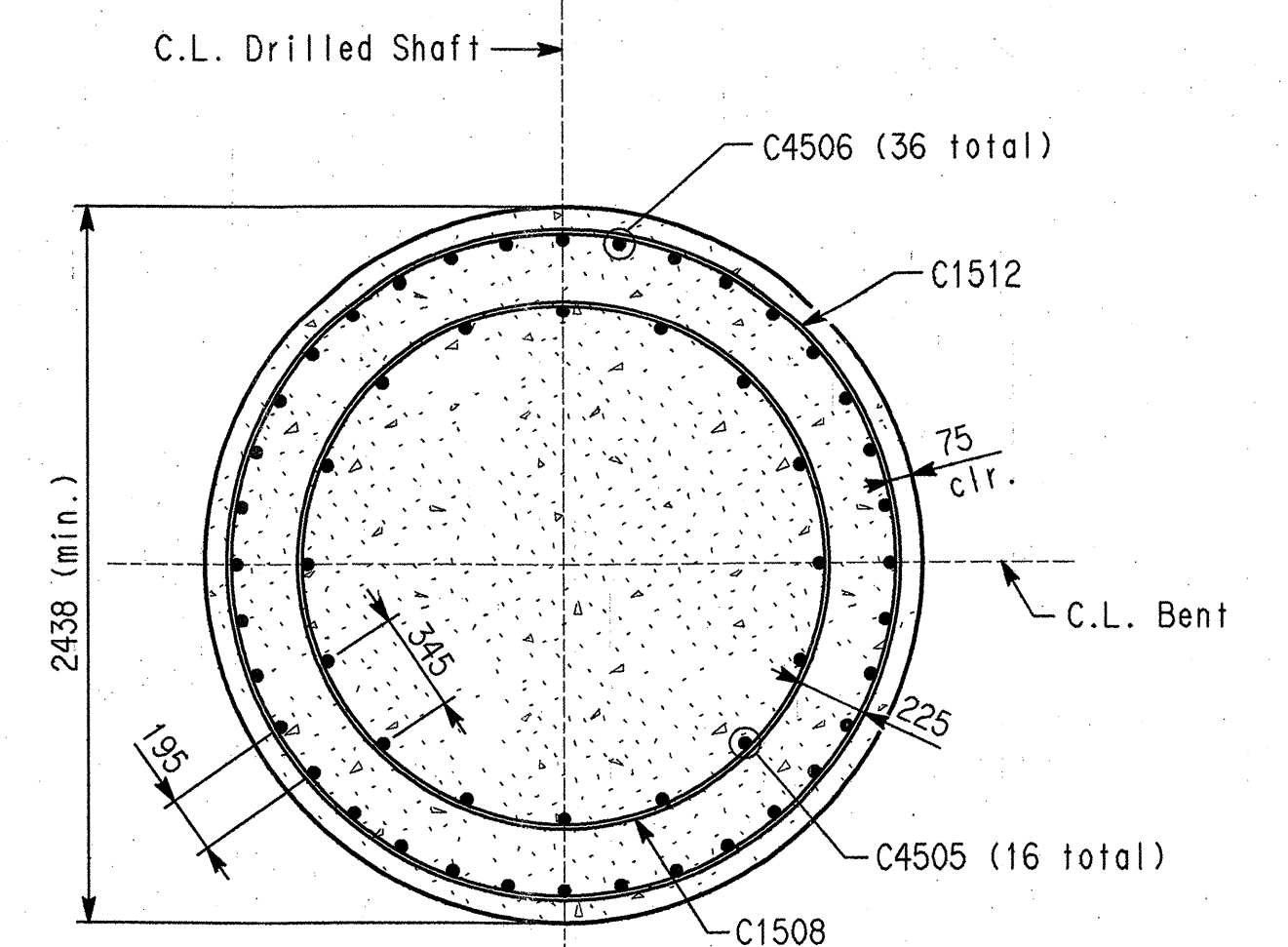
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SECTION A-A
Scale 1:25

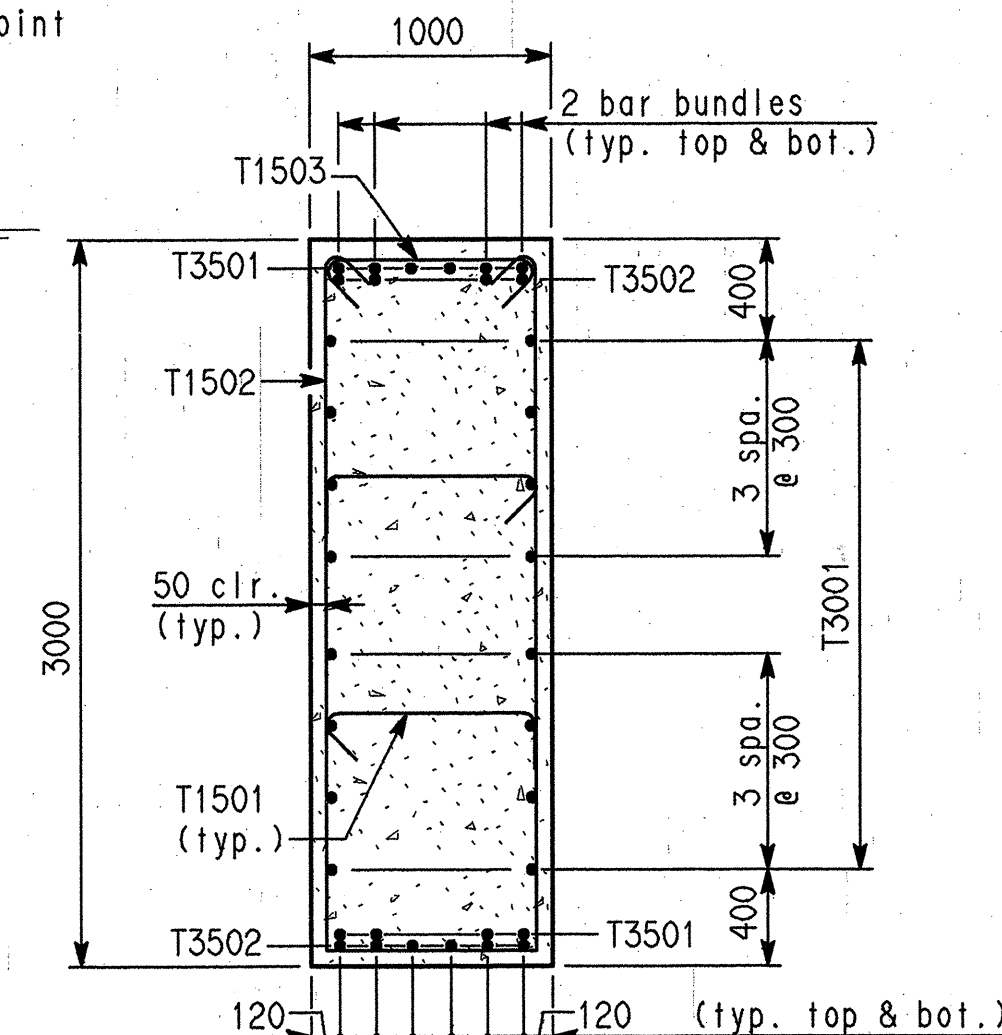


SECTION B-B
Scale 1:25



SECTION C-C
Scale 1:25

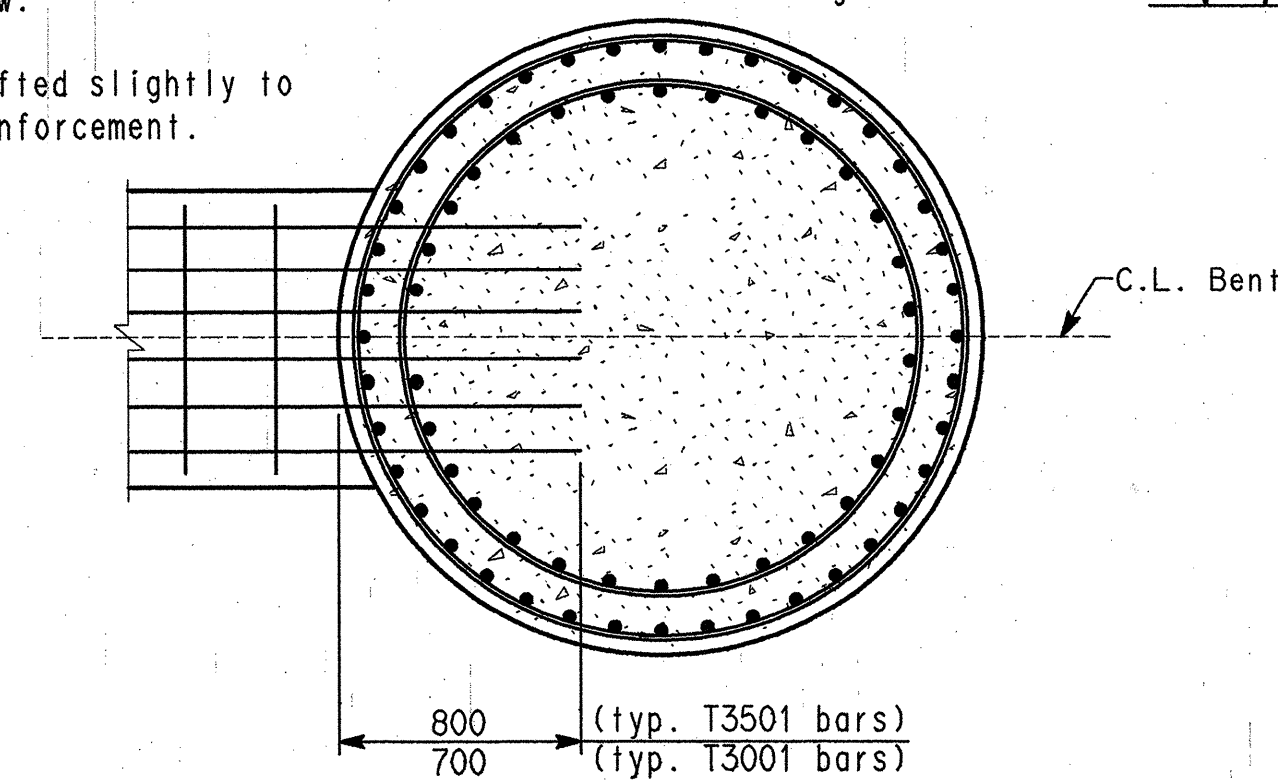
Note: Stay-in-Place form for drilled shaft not shown.



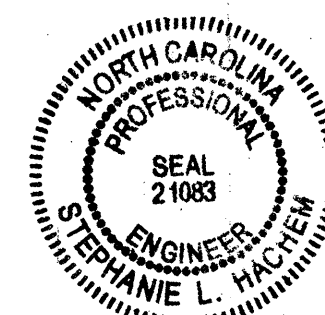
SECTION D-D
No Scale

Note: Alternate the 135° hook on each side of strut for the T1501 bars in each row.

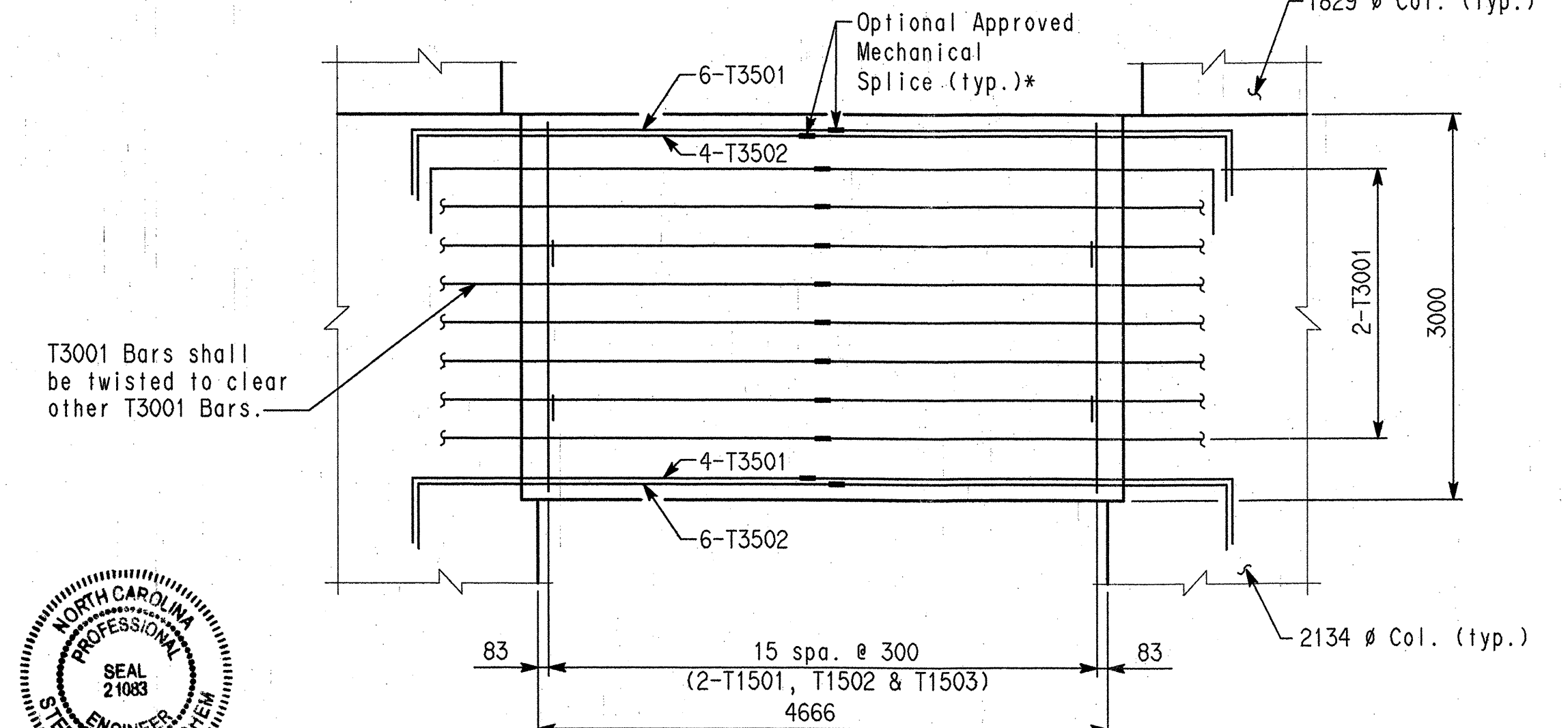
T3001 maybe shifted slightly to pass column reinforcement.



SECTION E-E
No Scale



Arkansas Temporary Permit Number 96-45,
Issued 12-15-96.
Signature of Holder *Stephanie L. Hadden*



STRUT ELEVATION
No Scale

*No extra payment will be made for optional mechanical splices.

NOTES
Stations and elevations are in meters (m).
All other dimensions are in millimeters (mm) unless otherwise noted.
Bent concrete shall be Class "S" with a minimum 28 day compressive strength $f'_c = 24$ MPa.
Concrete shall be poured in the dry and all exposed corners to be chamfered 20 unless otherwise noted.
Drilled Shafts concrete shall be Class "S" (modified) with a minimum 28 day compressive strength $f'_c = 28$ MPa.
All reinforcing steel shall conform to AASHTO M 31M or M 53M, Gr. 400 (yield strength = 400 MPa)
For additional information see layout.
For drilled shafts information see reinforcing bar list sheet.

All dimensions are in millimeters (mm) unless otherwise noted.

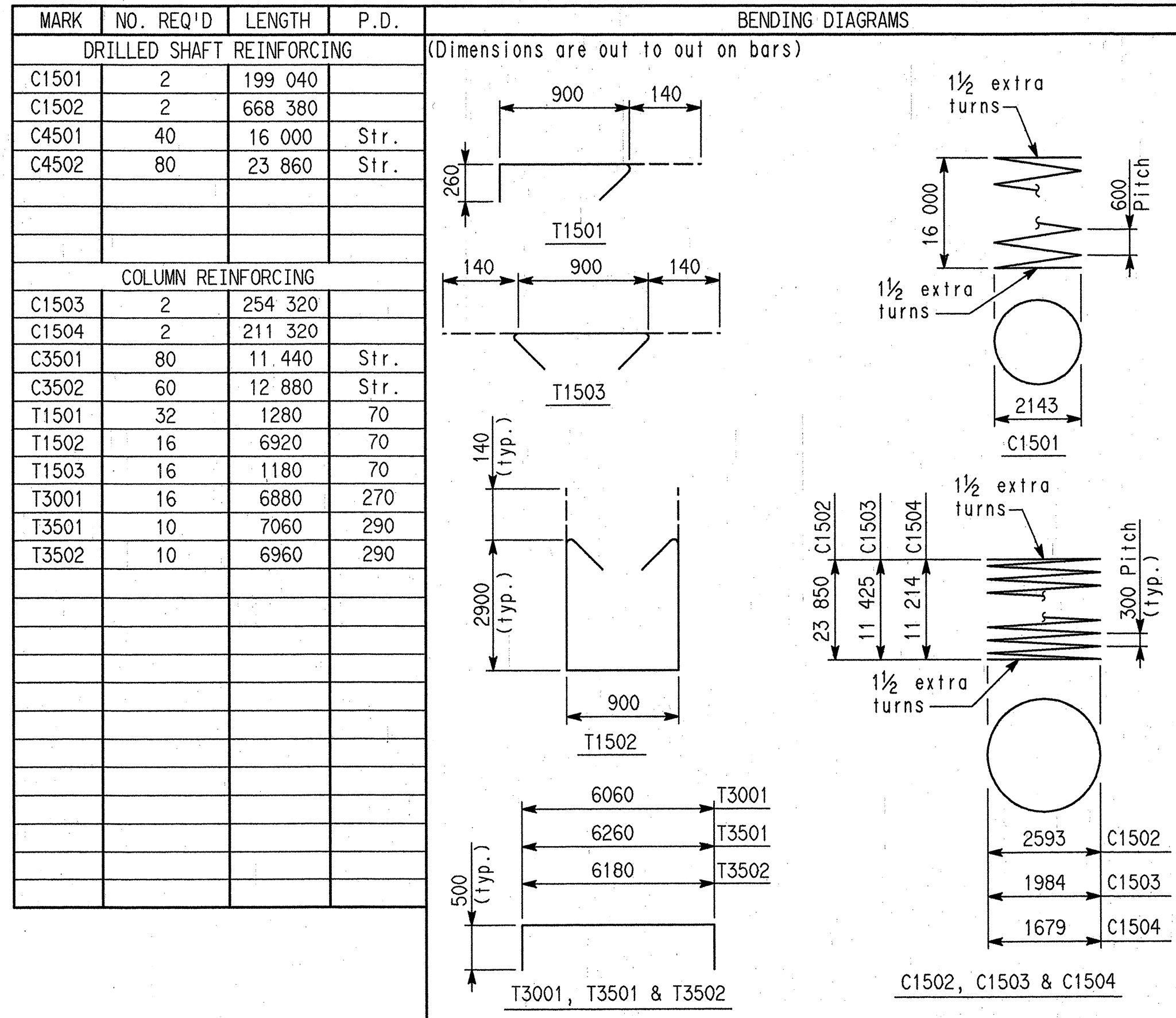
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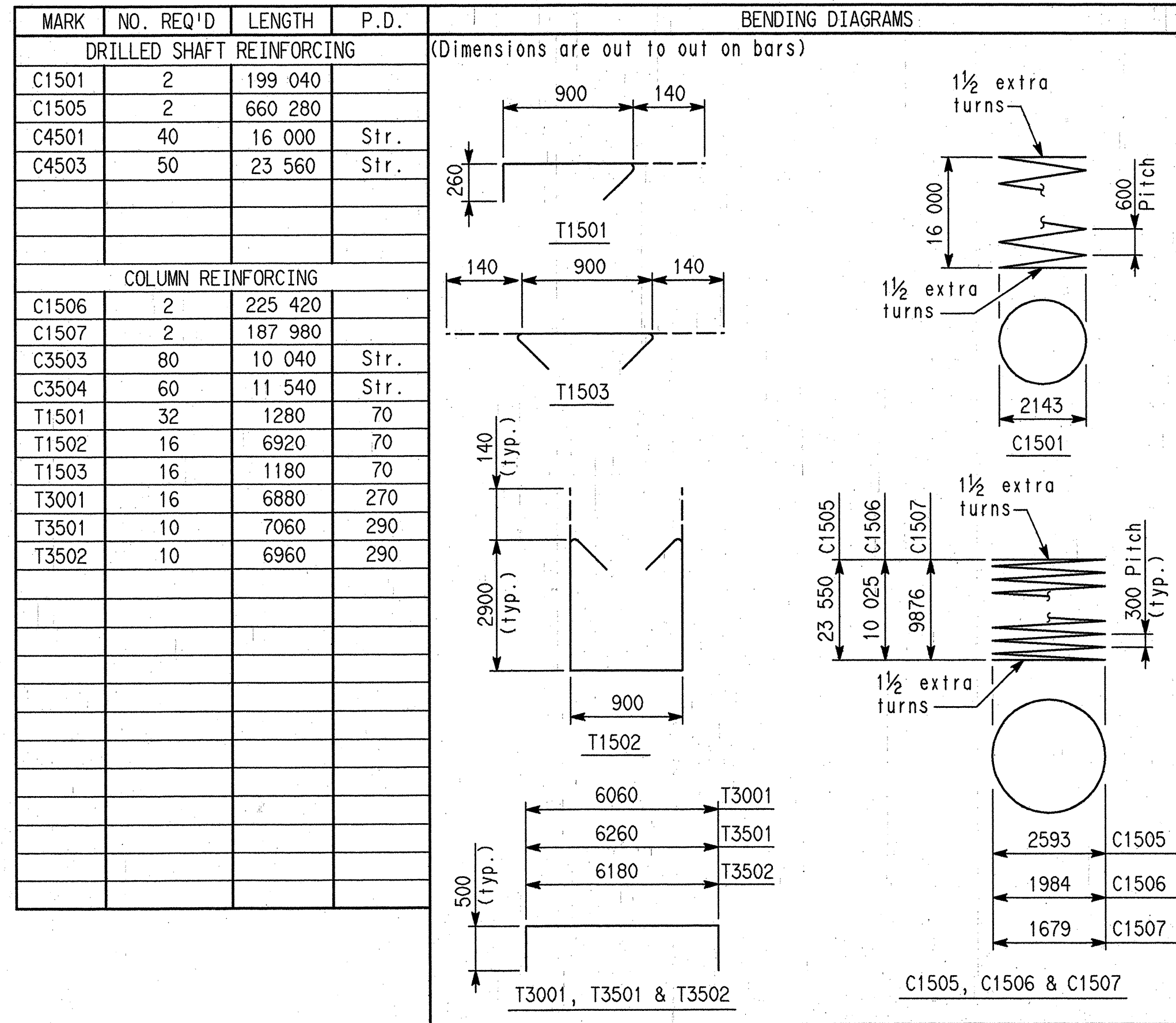
EASTBOUND BRIDGE
SHEET 1 OF 1
BENT 5
(COLUMNS)
US. HWY. 412
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARKANSAS

DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted
DESIGNED BY: SLH DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 39250

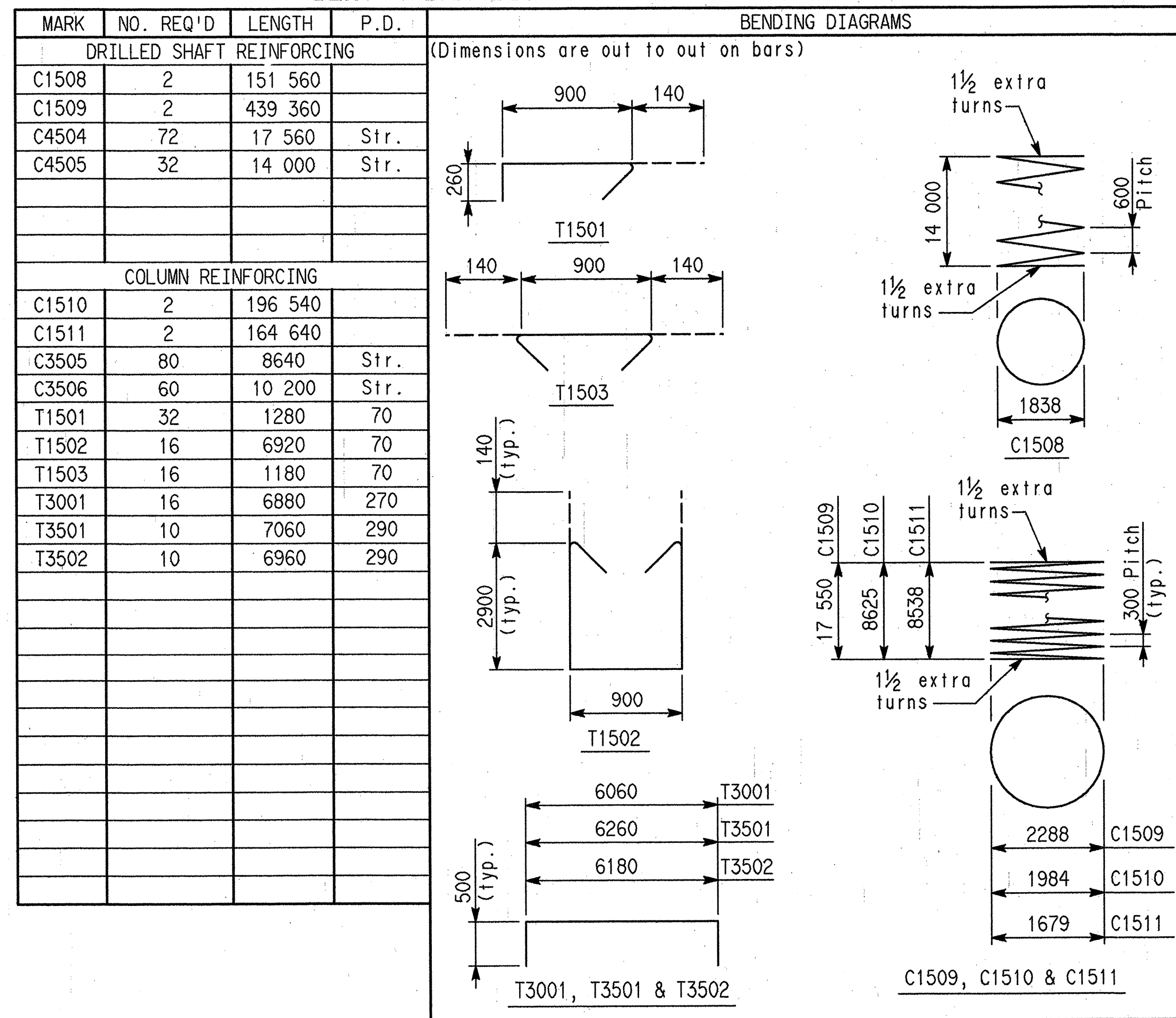
BENT 2 BAR LIST AND BENDING DIAGRAMS



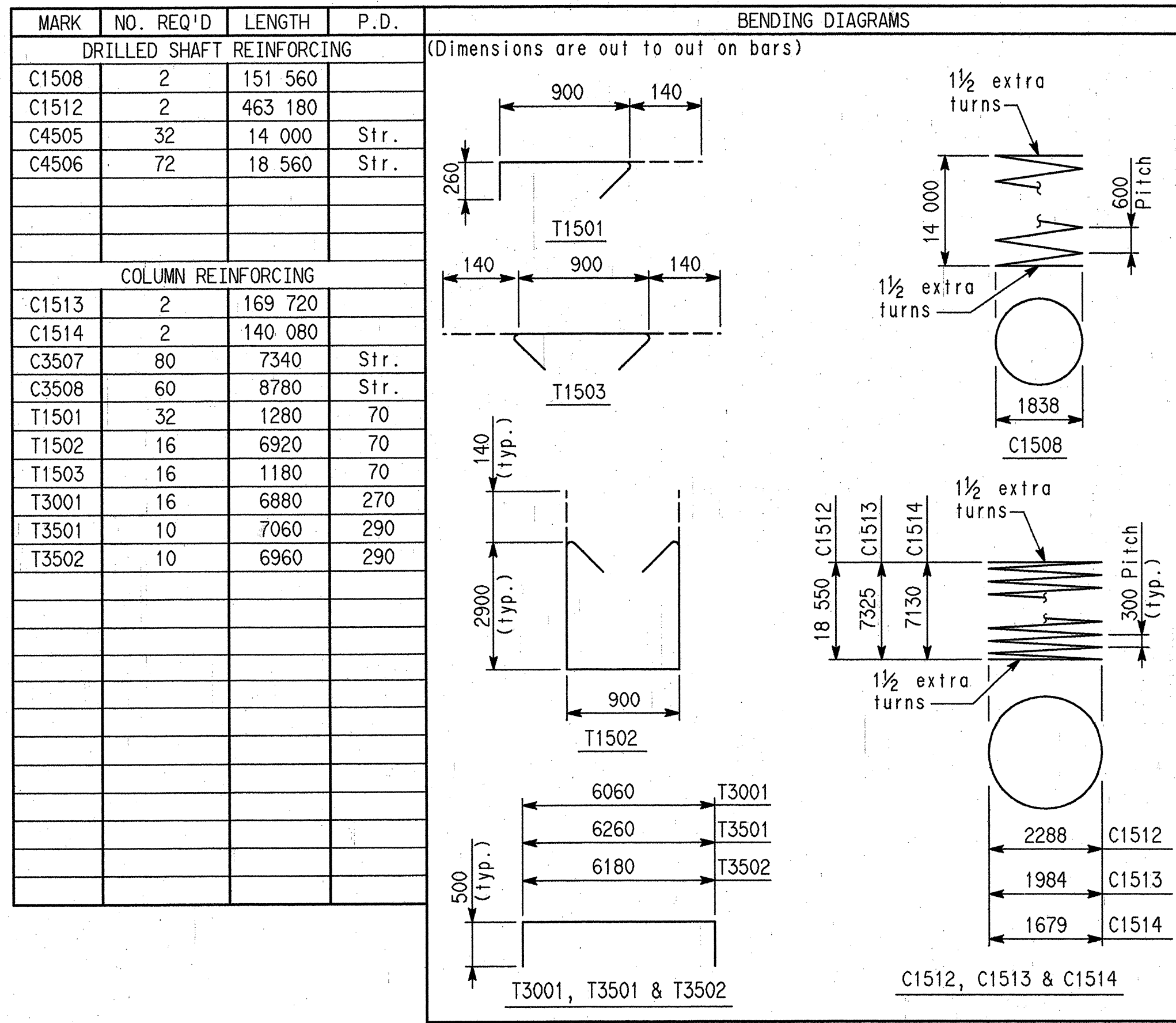
BENT 3 BAR LIST AND BENDING DIAGRAMS



BENT 4 BAR LIST AND BENDING DIAGRAMS



BENT 5 BAR LIST AND BENDING DIAGRAMS



DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	Fed Road Dist. No.	State	Fed. and Proj No.	Sheet No.	Total Sheets
7-14-97	7-23-97				Ark.		27	118
						Job No. 040236		
						B6686 BENT BAR LIST		38254-39251

Notes on Drilled Shafts:

For drilled shafts see special provisions.

Any parts of the permanent steel casing shall be removed, or the concrete shall be placed by use of forms, to a depth of 0.7m below the water level. Curing, stripping, and finishing shall be the same as for other structural concrete.

Construction of the columns shall proceed on the drilled shafts.

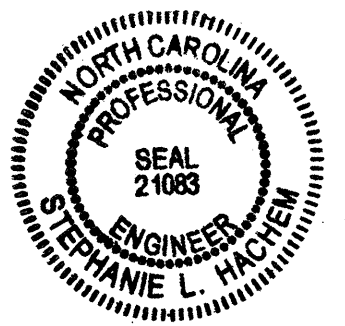
The design assumes material that will provide a minimum bearing capacity of 4.0 MPa per caisson. The caissons for Bent 2 and Bent 3 shall be drilled into the material designated as Hard Dark Gray Shale formation on the boring legend. The caissons for Bent 4 and Bent 5 shall be drilled into the material designated as Very Hard Light Gray Limestone on the boring legend. The minimum depth of embedment into the respective formations shall be 2 times the diameter of the shaft. (The expected tip elevations are shown in the plans; however, the above criteria must be met.)

Lengths of drilled shafts shown are for estimating quantities and for use in determining payment in accordance with the special provision.

Beaver Lake is the water source for the Beaver Water District, and the contractor cannot discharge excavated material or materials involved with drilling the shafts into the lake. Drilled shaft construction shall be in accordance with Section 110.07 of the 1996 standard specifications.

To facilitate construction, the contractor may drill a larger caisson to the beginning of the competent rock formation then reduce the diameter to the dimensions shown on the plans. If the maximum shaft diameter exceeds 3.048m, approval of the Engineer is required. A minimum of the diameter shown in the plans must be maintained. No additional costs will be paid for this option.

Drilled shaft reinforcing will be included in pay item for Drilled Shaft. The quantities shown in the Drilled Shafts are for informational purposes only. The quantities are not included in the totals shown on the Schedule of Bridge Quantities sheet.



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Issued 12-15-96.
Signature of Holder *Stephanie L. Hackett* 2/1/97

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**EASTBOUND BRIDGE
SHEET 1 OF 1
BENT 2 THRU 5
REINFORCING BAR LIST**

US. HWY. 412
**ARKANSAS STATE HIGHWAY
COMMISSION**

LITTLE ROCK, ARKANSAS

DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: No Scale
DESIGNED BY: SLH DATE: 12/96

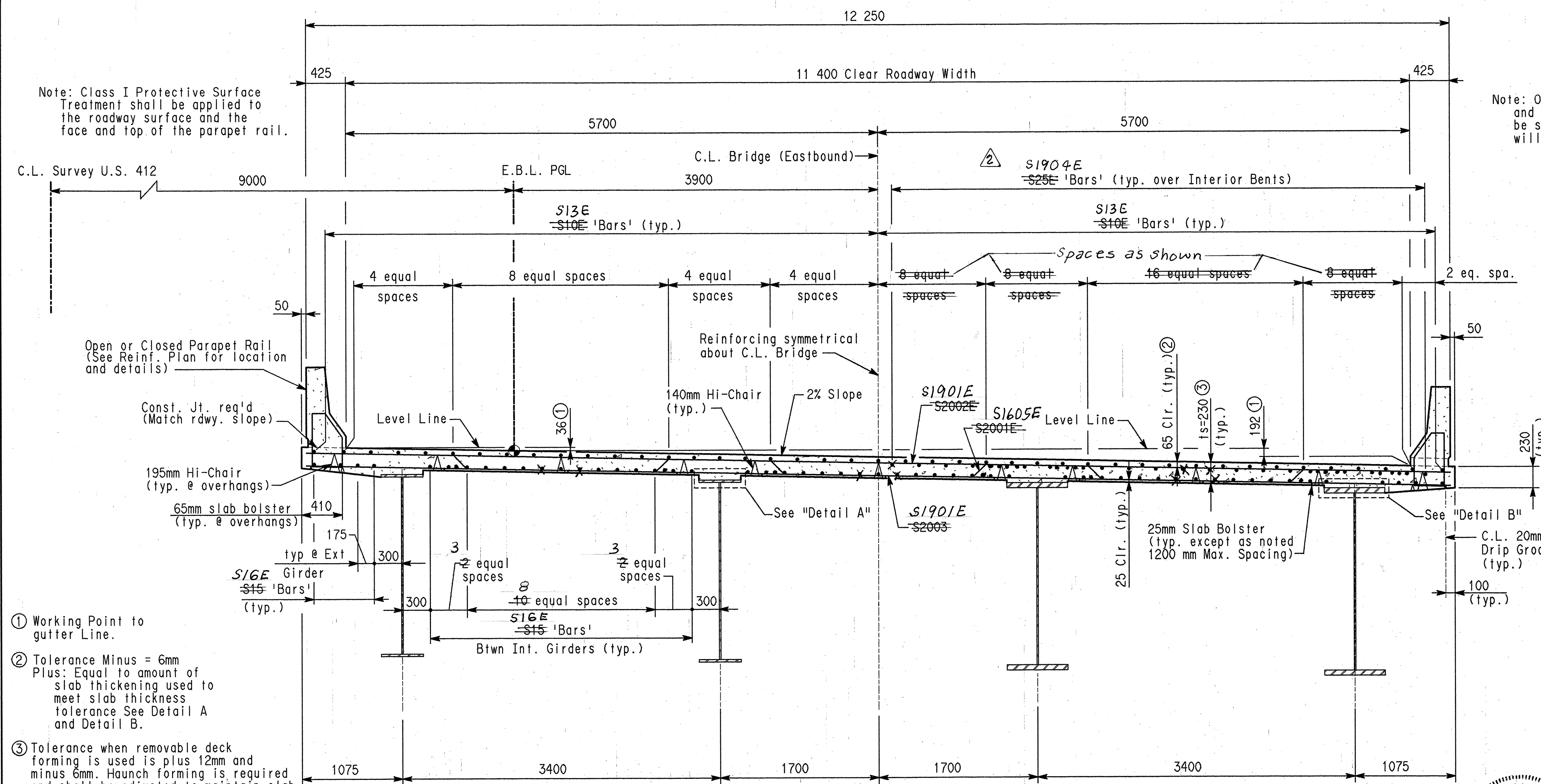
BRIDGE NO. B6686

DRAWING NO. 39251

Rev. dwg. no. WRR 7-14-97

All dimensions are in millimeters (mm) unless otherwise noted.

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	Fed Road Dist. No.	State	Fed. and Proj No.	Sheet No.	Total Sheets
7/14/97	7-23-97				Ark.		30	118
12/4/98	2-11-99			Job No.		040236		
①				B6686	TYP. SECTION		30254 39254	



(typ.)

① Working Point to gutter Line.

② Tolerance Minus = 6mm
Plus: Equal to amount of slab thickening used to meet slab thickness tolerance See Detail A and Detail B.

③ Tolerance when removable deck forming is used is plus 12mm and minus 6mm. Haunch forming is required and shall be adjusted to maintain slab thickness tolerance. Haunch dimension may vary within the following limits to maintain the grade and slab thickness tolerance.

Minimum - occurs when top flange contacts bottom reinforcing steel.

Maximum - top flange thickness plus 44 mm

No adjustment for increase in quantities will be made for thickening slab of deepening haunch.

See Std. Dwg. No. 36515 for tolerances when permanent steel deck forms are used. "Payment for concrete shall be based on removable deck for

1075

Notes

All structural steel shall be AASHTO M270, Gr. 345W unless otherwise noted, and it shall be paid for at the unit price per Kilogram bid for "Structural Steel in Plate Girder Spans". AASHTO M270, Gr. 345W shall not be painted. All exposed surfaces to be cleaned in accordance with Subsection 807.84(e) of the Standard Specifications and the Job Special Provision.

Web and flange plates are considered main load carrying members and shall meet the Longitudinal Charpy V-Notch Test specified in Section 807.05 of the Standard Specifications.

Steel Plates for main members and flange field splice plates shall be cut and fabricated so that the primary direction of rolling is parallel to the direction of the main tensile and/or compressive stresses.

All girders shall be blocked in their true position in the shop, in groups of a minimum of three sections. Girders shall be blocked with webs horizontal. See Section 807.54(b)(2) of the Standard Specifications. The camber length of sections, distance between bearings, and openings of joints shall be measured with the girders in their true position and this information shall become a part of the permanent records of this job. The component parts shall be match marked in this assemble and these marks shall be shown on the erection diagram. All girder dimensions are based on a temperature of 15 degrees C. A tolerance of 6mm is allowed for camber.

Girder webs may be made by shop splicing with a minimum length of 7.5m for sections. Flange plates longer than 15m may be made by shop splicing with a minimum length of 7.5m for sections. No additional payment for these welds will be made.

Elastomeric Bearings shall be firmly seated in accordance with subsection 808.08 of the specifications. This work to be considered as subsidiary to the Item "Elastomeric Bearings" and will be paid for directly.

Structural shapes of equal or greater strength may be substituted for shapes shown if approval is obtained from the Bridge Engineer. Payment will be made on the basis of shapes shown.

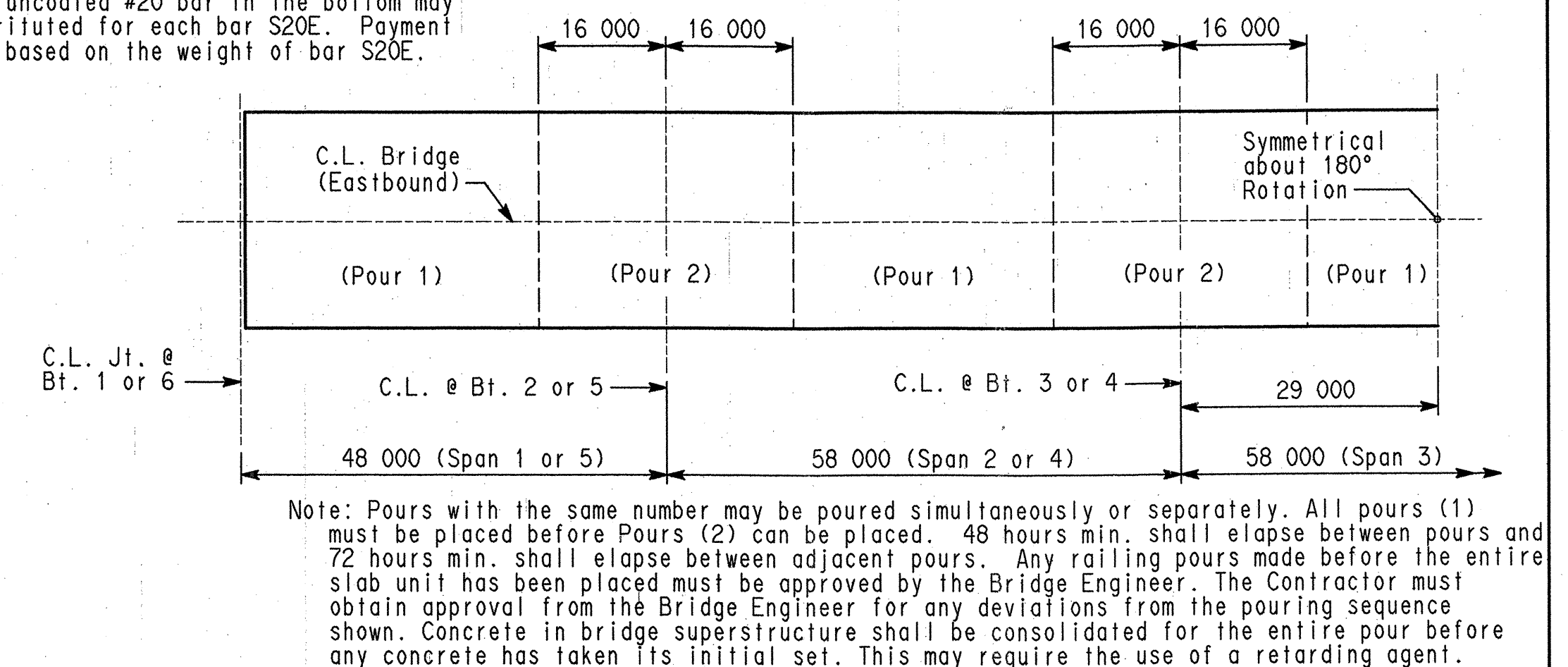
TYPICAL ROADWAY SECTION

Scale 1:30

over Interior Bents

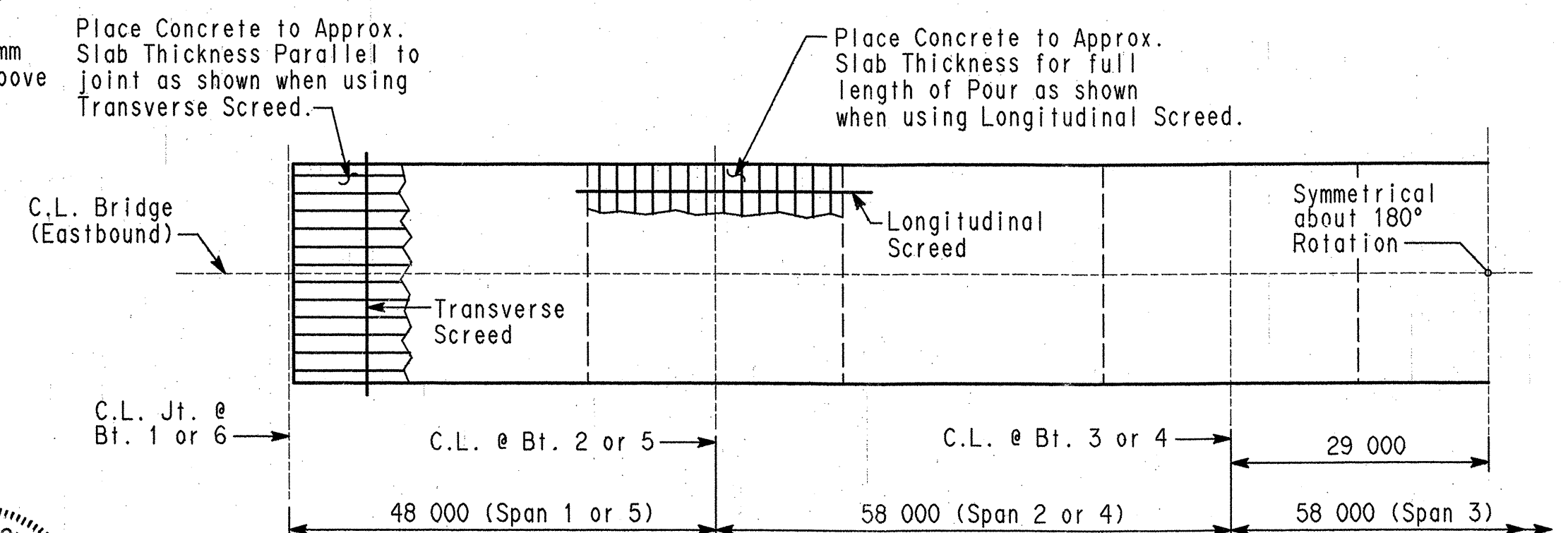
Arkansas Temporary Permit Number 96-45,
Issued 12-15-96.
Signature of Holder Shirley H. 2/2/97

Note: One epoxy coated #20 bar in the top and one uncoated #20 bar in the bottom may be substituted for each bar S20E. Payment will be based on the weight of bar S20E.



SLAB POURING SEQUENCE

No Scale



CONCRETE PLACEMENT PROCEDURE

No Scale

Load Distribution to Girder:

Dead load

To Interior Girder

$$22.210 \text{ kN/M} + 1.1 \text{ (Wt./M of Girder)}$$

To Exterior Girder

$$18.021 \text{ kN/M} + 1.1 \text{ (Wt./M of Girder)}$$

Live load to Composite Girder

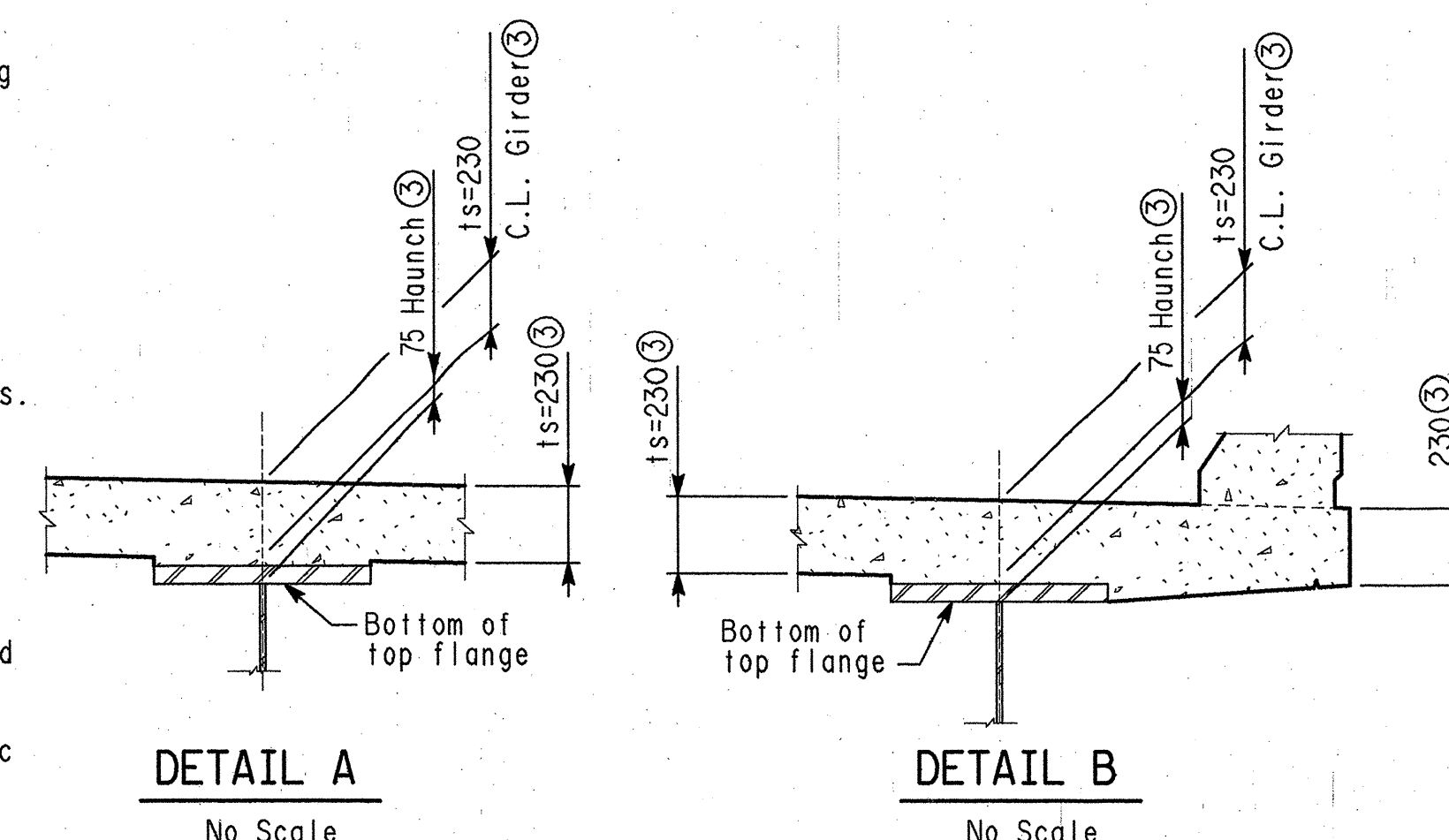
2.028 Wheels + Impact

1.643 Wheels + Impact

* (Includes 2.850 kN/M for future wearing surface)

Reinforcing Steel:

The reinforcing steel shall be accurately located in the forms and firmly held in place by steel wire supports sufficient in size and number to prevent displacement during the course of construction. The wire supports will not be paid for directly but will be considered subsidiary to the item of "(Epoxy Coated Reinforcing Steel Grade 400)".



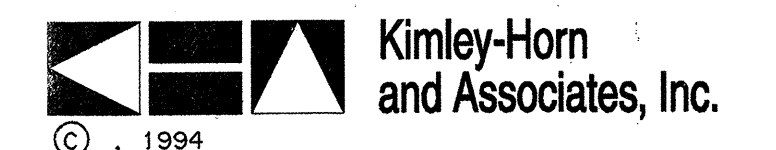
DETAIL A

No Scale

DETAIL B

No Scale

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EASTBOUND BRIDGE
SHEET 1 OF 5
270m CONTINUOUS PLATE
GIRDER (TYPICAL SECTION)
US HWY 412

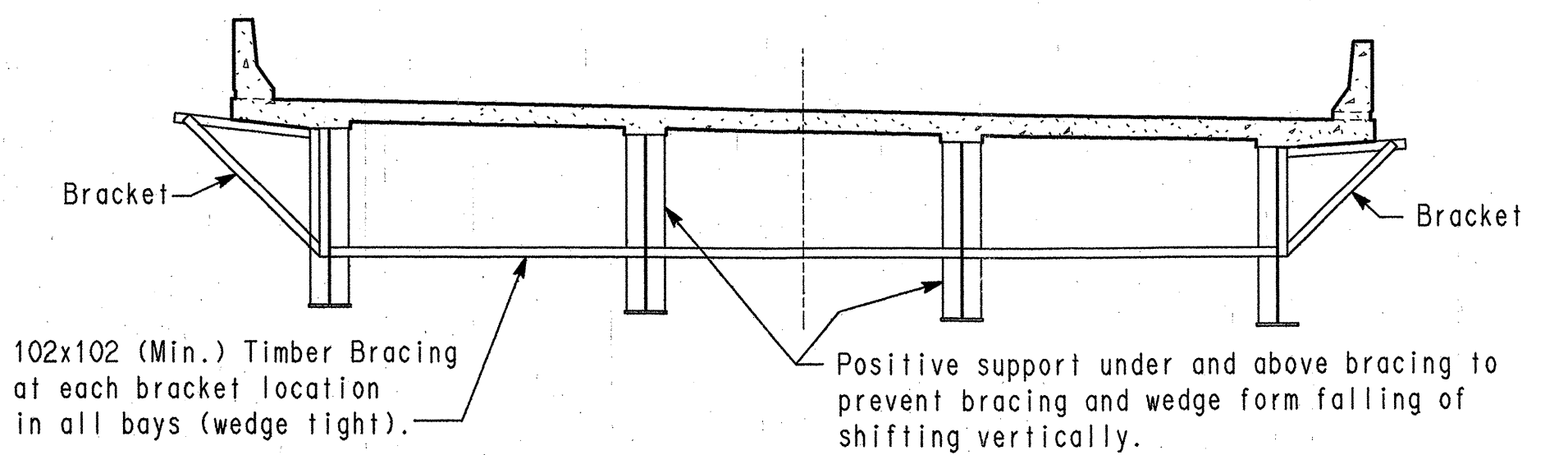
ARKANSAS STATE HIGHWAY
COMMISSION

LITTLE ROCK, ARKANSAS

DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted

DESIGNED BY: SLH DATE: 12/98
BRIDGE NO. B6686 DRAWING NO. 30254 39254

①



Camber for Dead Load Deflection plus Vertical curve $\pm 6\text{mm}$ tolerance.
Negative sign (-) indicates upward deflection.

MICROFILMED
MAY 12 1997

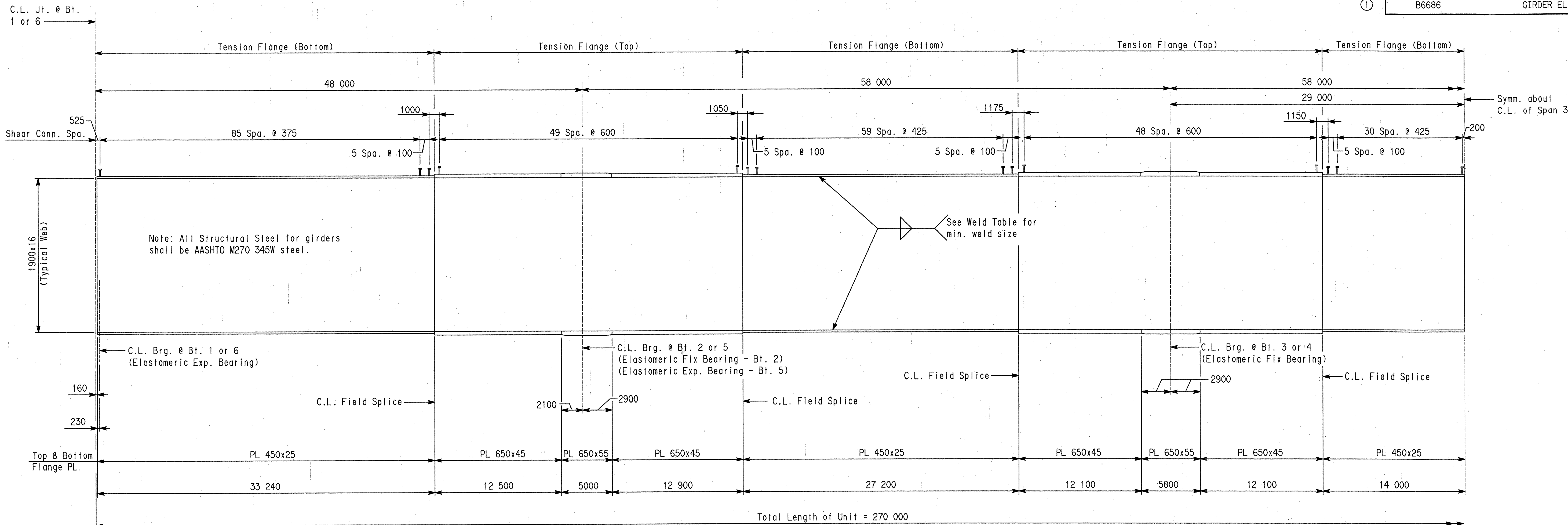


SCREED RAIL SUPPORT
Scale NTS

s:\203400\dgn\east\fpea01.dgn
06 FEB 97

09:00:01
IRELAND

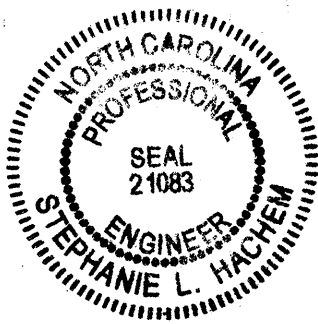
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	Fed Road Dist. No.	State	Fed. and Proj No.	Sheet No.	Total Sheets
7/14/97	7-23-97				Ark.		32	118
				Job No.		040236		
				B6686		GIRDER ELEV.		39256 39256



Note: For details of Elastomeric Bearings, see drwg. no. 38263.39263

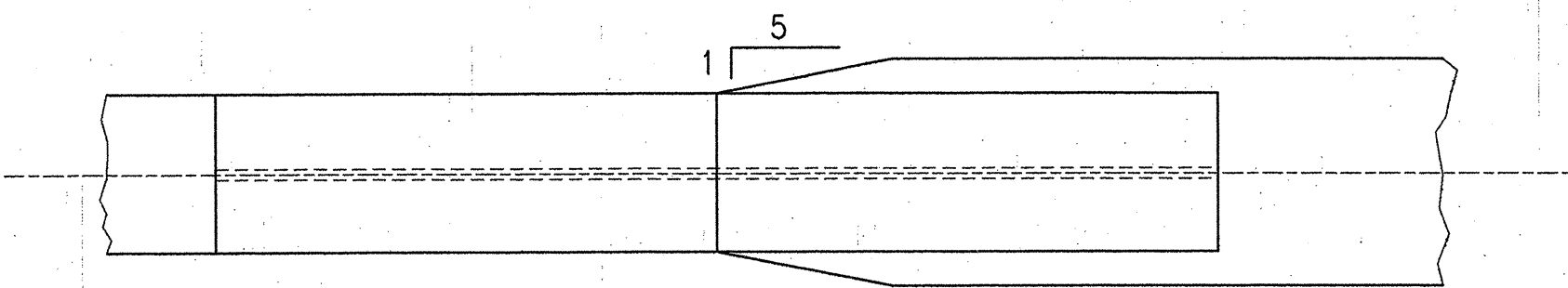
TYPICAL GIRDER ELEVATION

Not to Scale



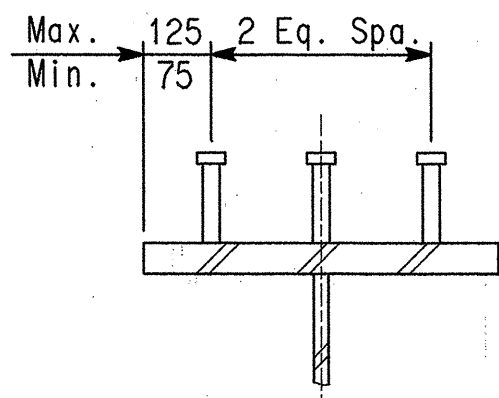
Arkansas Temporary Permit Number 96-45,
Issued 12-15-96.
Signature of Holder *[Signature]* 7/17/97

Note: For Structural Steel notes see Typical Roadway Section.



TOP FLANGE SPLICE

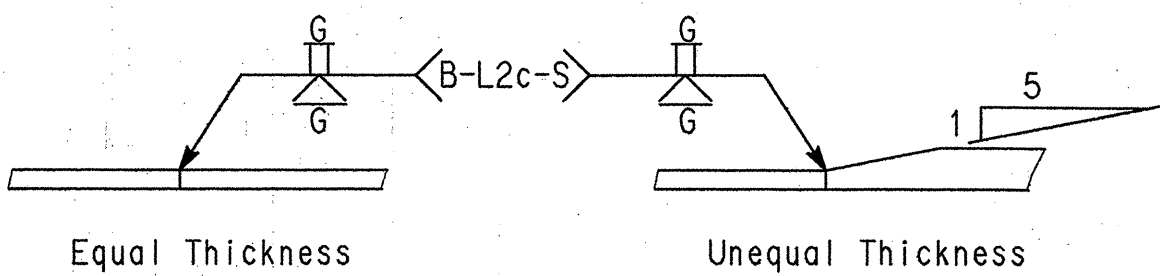
No Scale



SHEAR CONNECTOR DETAIL

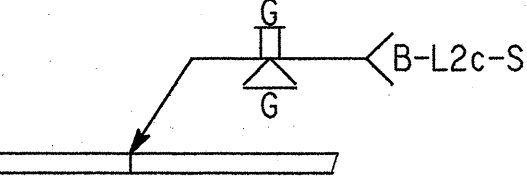
No Scale

Stud Shear Connectors shown shall be 22x127 Long, granular flux filled, solid fluxed or equal, and automatically end welded to the beam flange in accordance with the recommendations of the Manufacturer. 20# studs may be used in place of the 22# studs shown, at the ratio of 1.361-20# studs in place of one 22# stud. 22# studs will be used as basis for measurement of structural steel in shear connectors. Maximum stud spacing = 600mm.



FLANGE SPLICE

No Scale



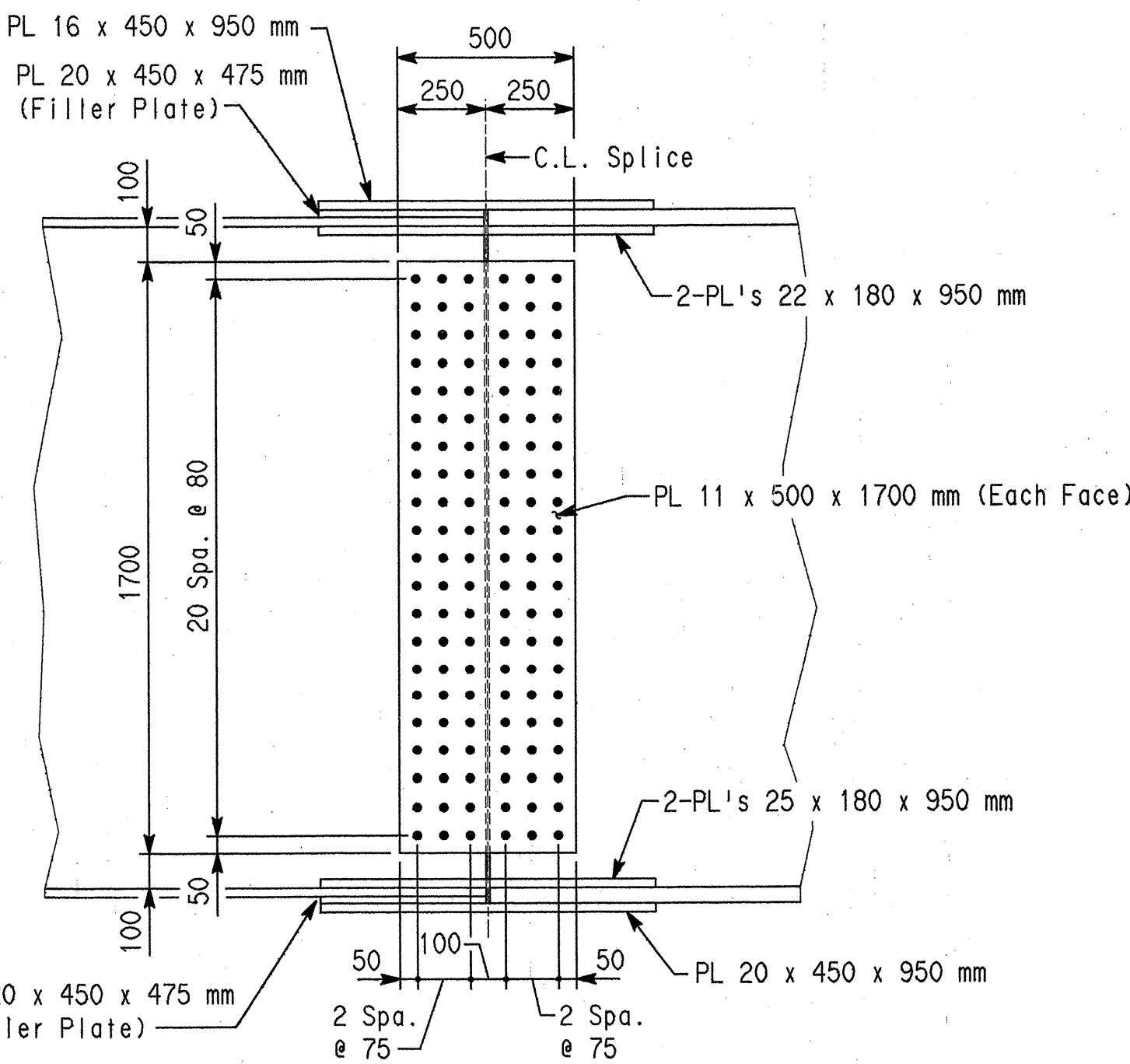
WEB SPLICE

No Scale

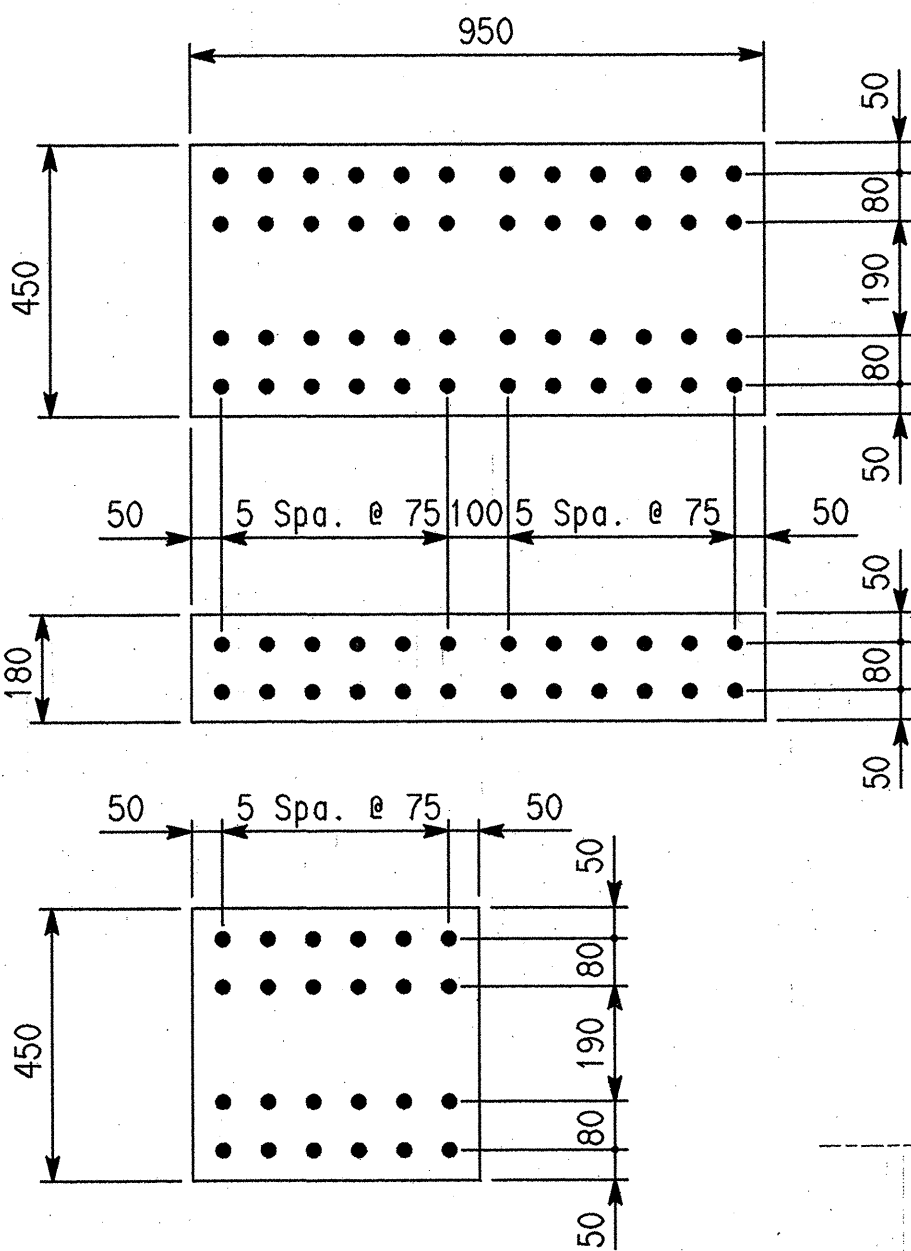
DETAILS OF WELDED SPLICES

No Scale

All dimensions are in millimeters (mm) unless otherwise noted.



(WEB SPLICE)



(FLANGE SPLICE)

DETAILS OF FIELD SPLICE

No Scale

- NOTES:
- All Field Splice plates shall be AASHTO M270 345W steel.
 - All Field Splice bolts shall be M22 H.S. bolts.

Rev. dwg. no. WRR 1-14-97

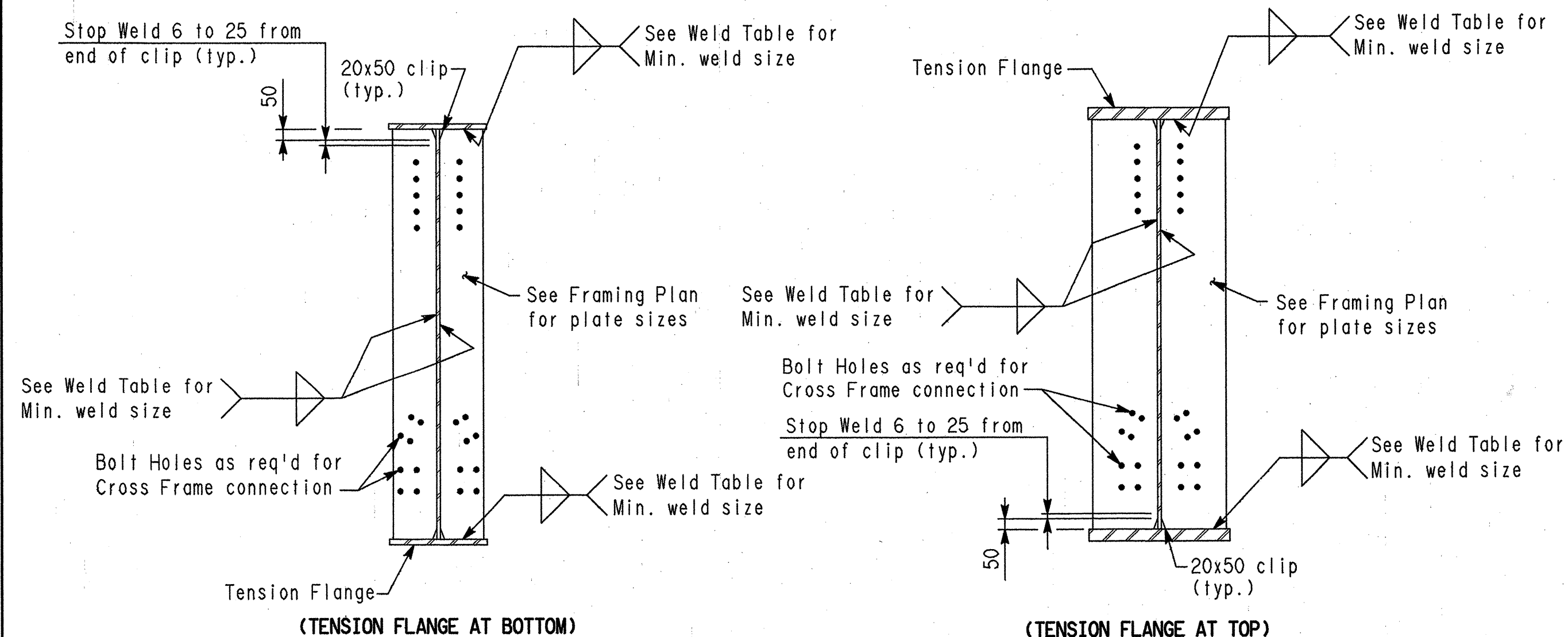


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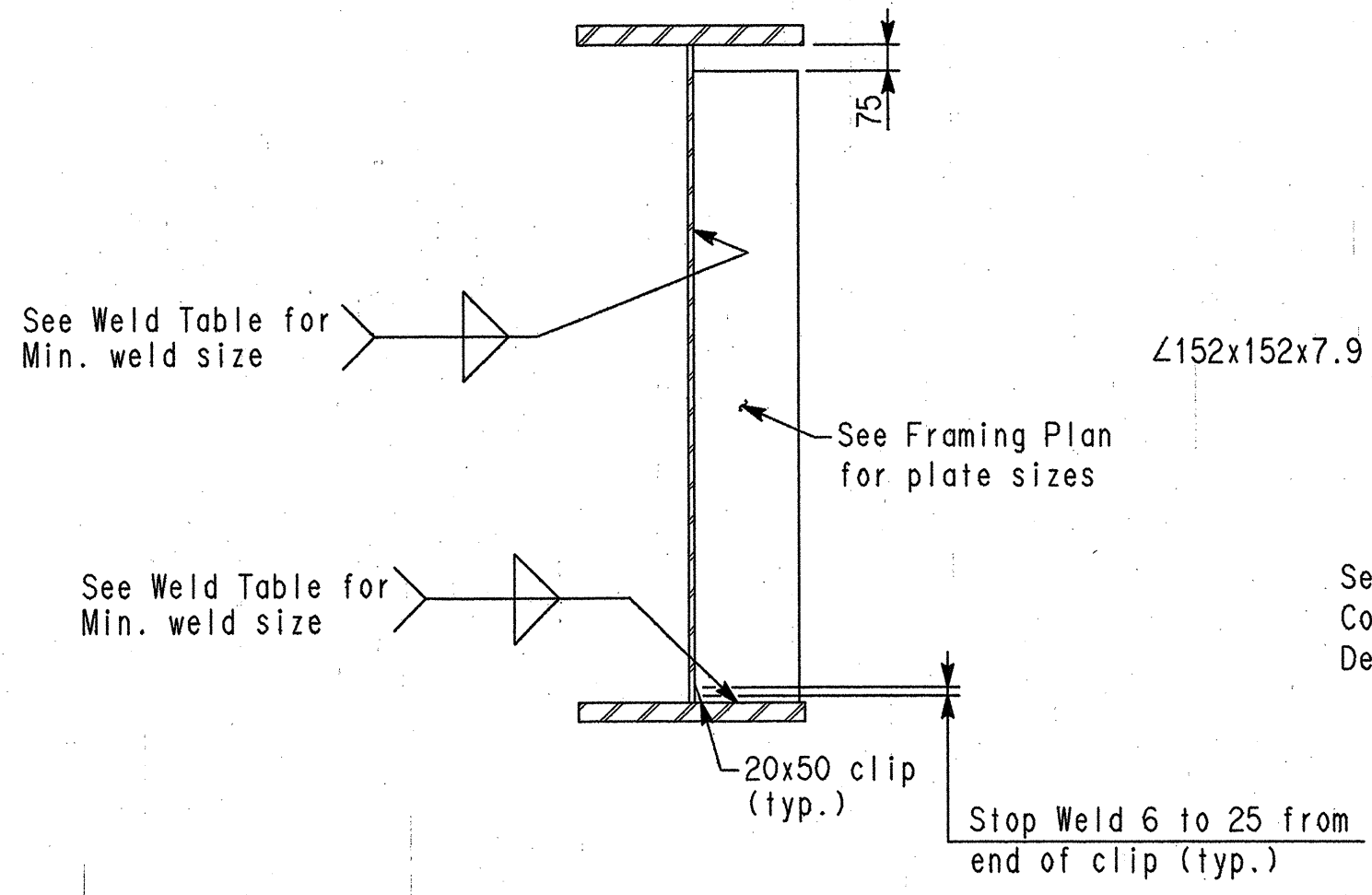
EASTBOUND BRIDGE
SHEET 3 OF 5
270m CONTINUOUS PLATE
GIRDER (GIRDER ELEVATION)
US. HWY. 412
ARKANSAS STATE HIGHWAY
COMMISSION
LITTLE ROCK, ARKANSAS

DRAWN BY: TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted
DESIGNED BY: MLR DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 39256 39256

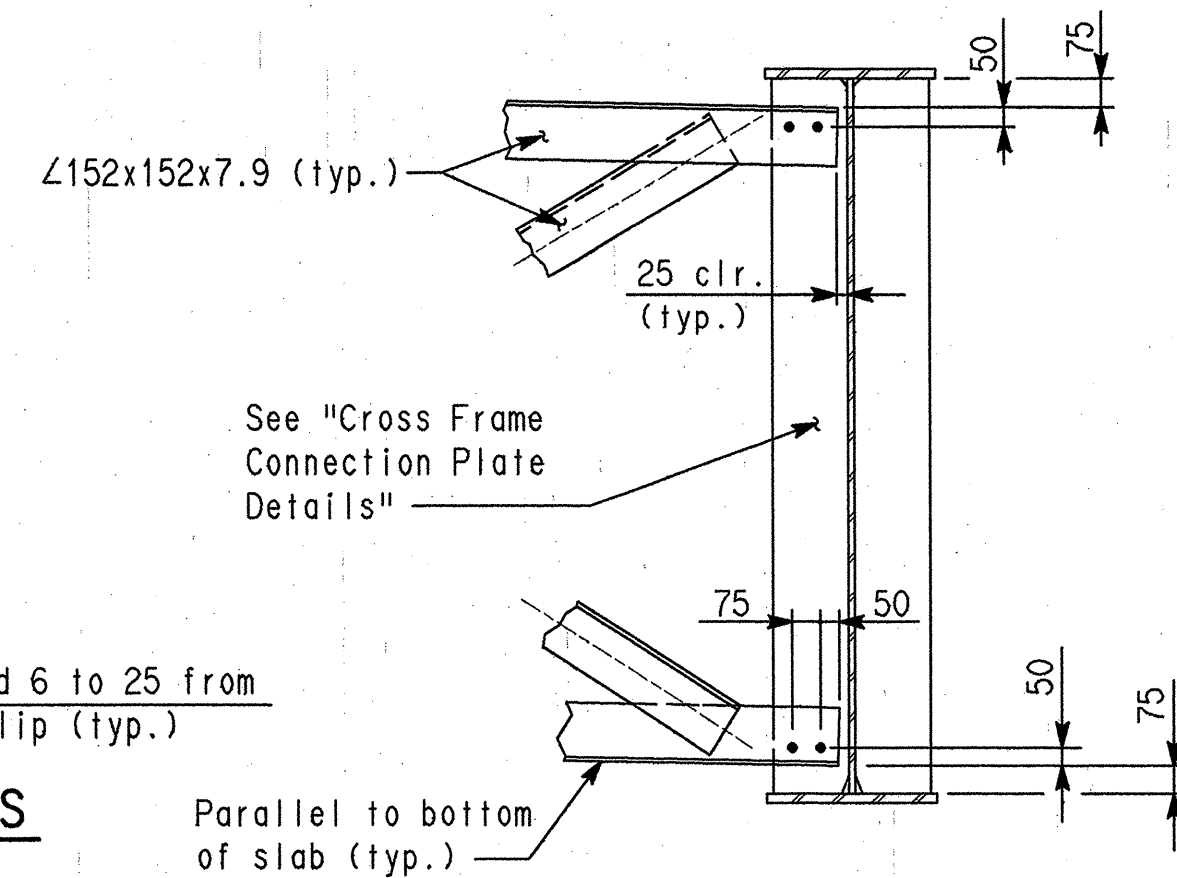
DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	Fed Road Dist. No.	State	Fed. and Proj No.	Sheet No.	Total Sheets
7-14-97	7-23-97				Ark.		33	118
				Job No.		040236		
				B6686		CROSS FRAME		38257 39257



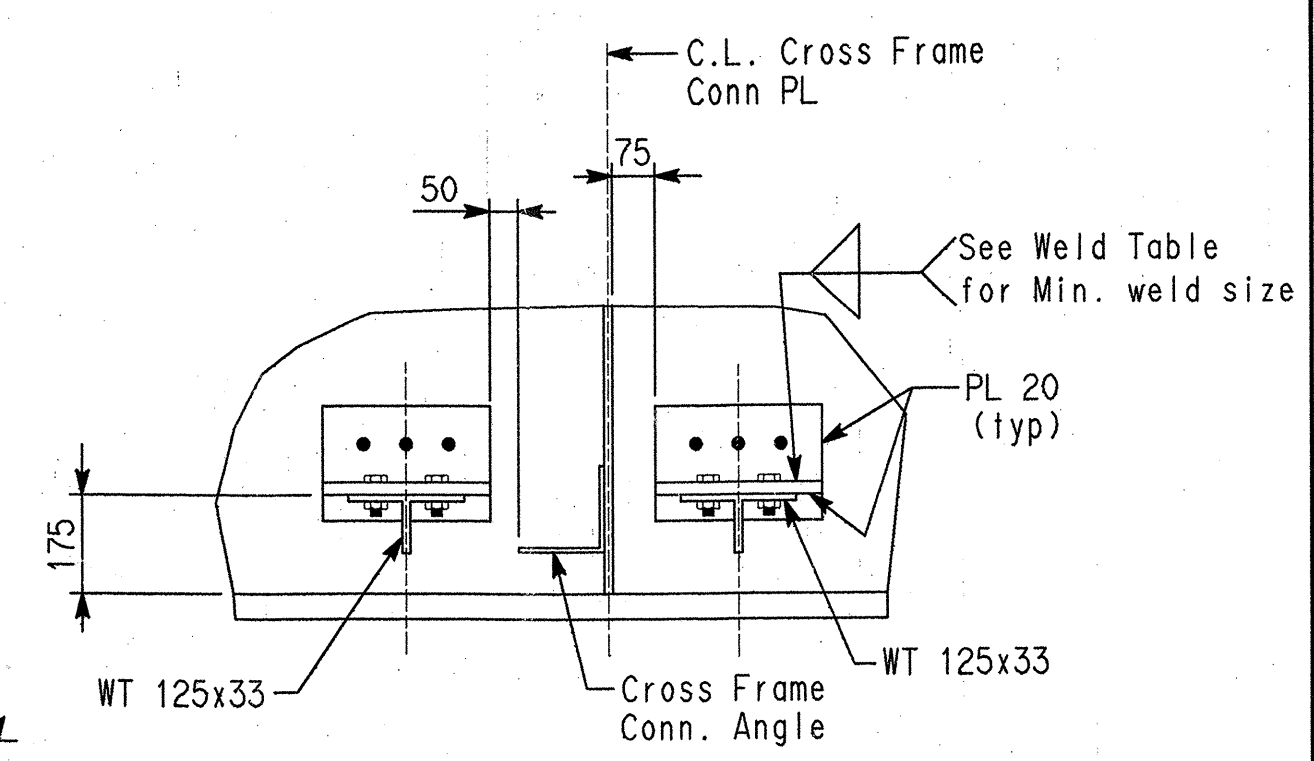
CROSS FRAME CONN. PLATE DETAILS
(Over Bents)



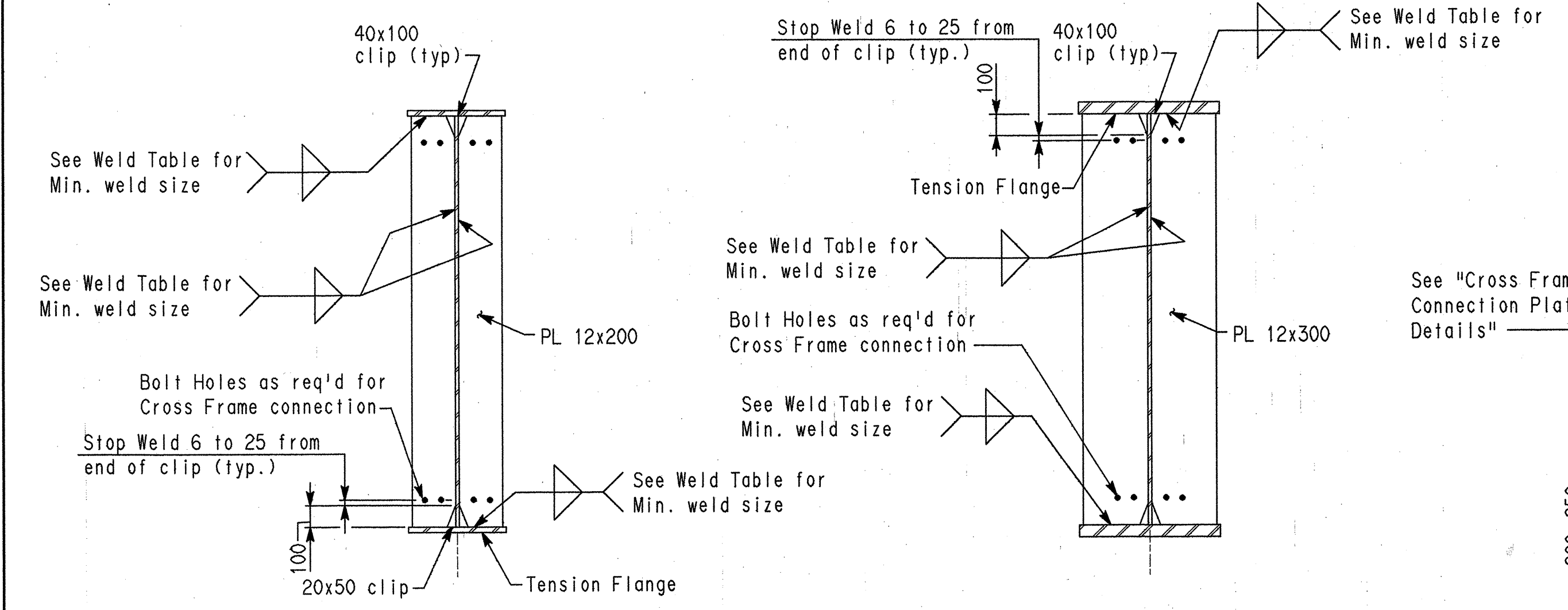
INTERMEDIATE STIFFENER DETAILS
(Tension Flange at Top)



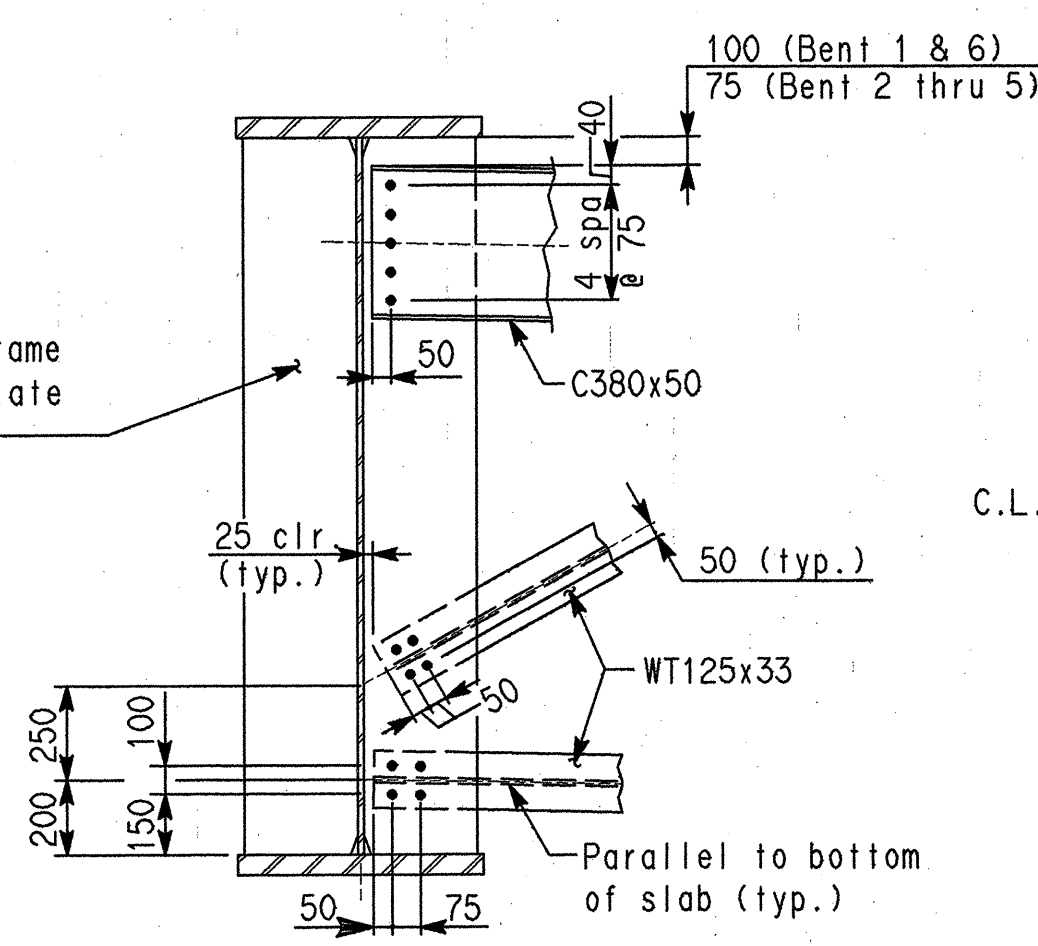
TYPICAL CROSS FRAME DETAIL
(Along Span)



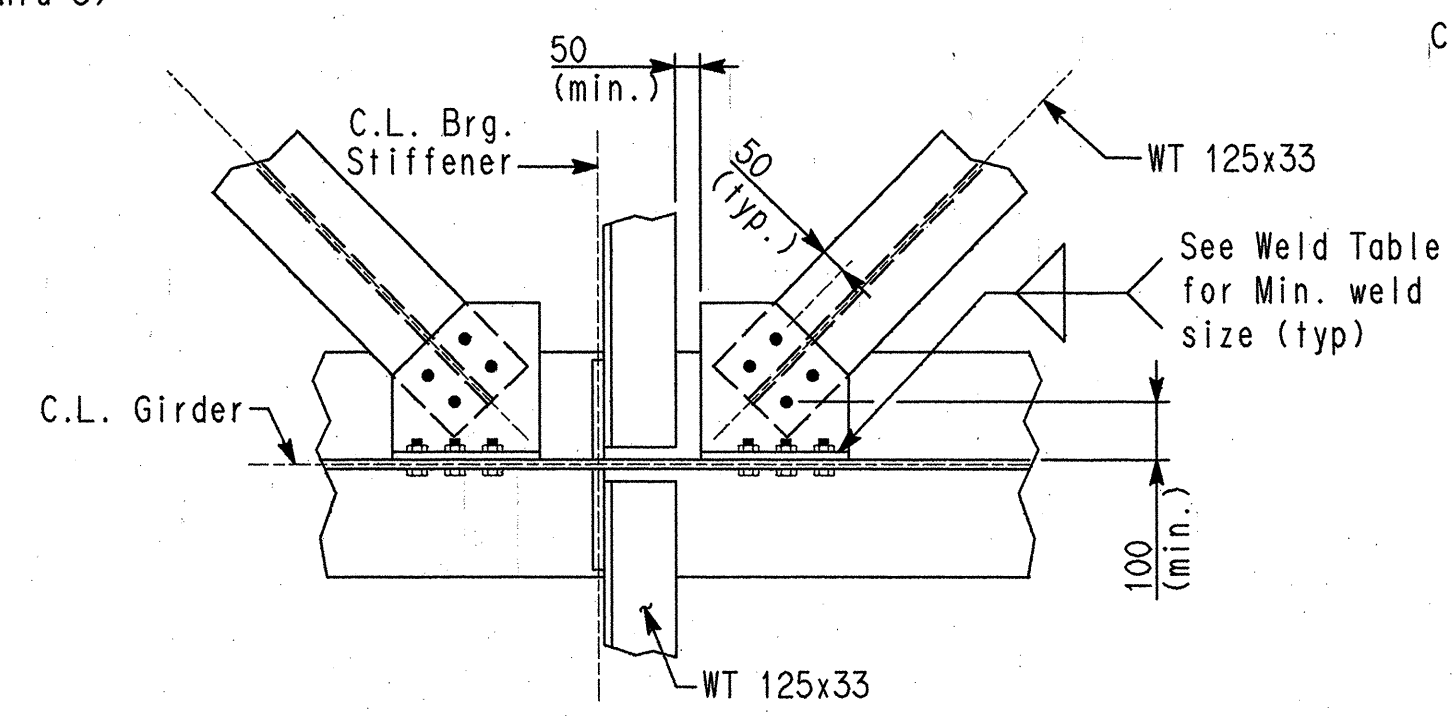
LATERAL BRACING
(Section thru Intermediate Cross Frame)



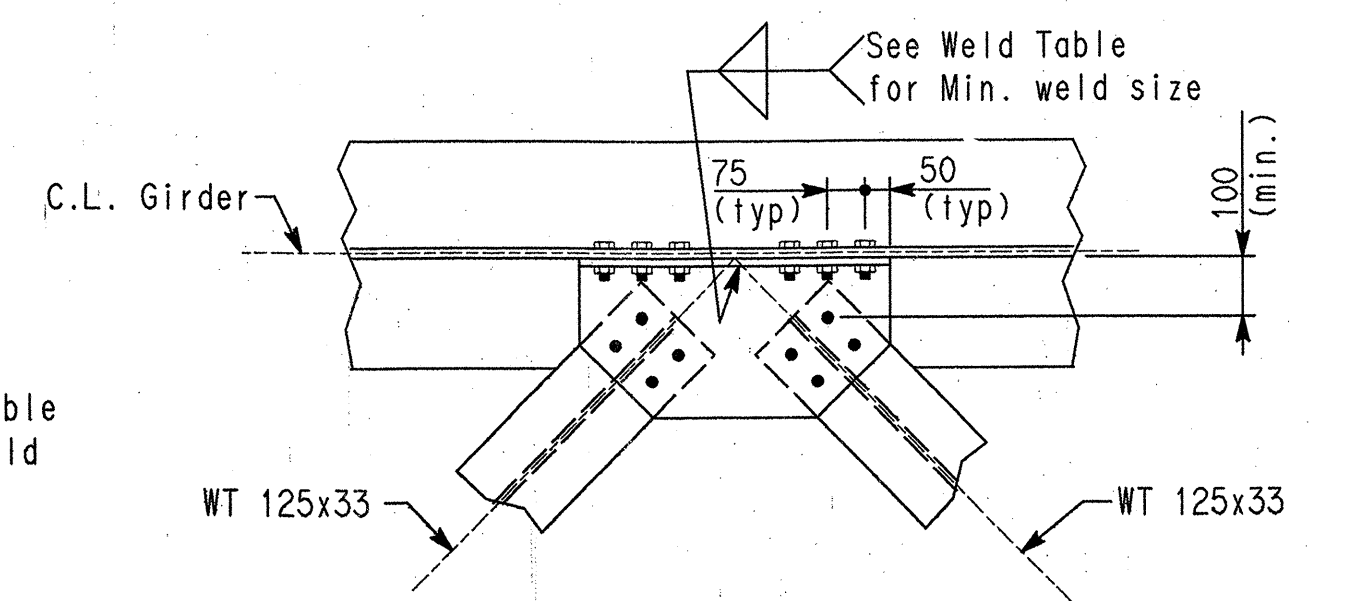
CROSS FRAME CONN. PLATE DETAILS
(Along Span)



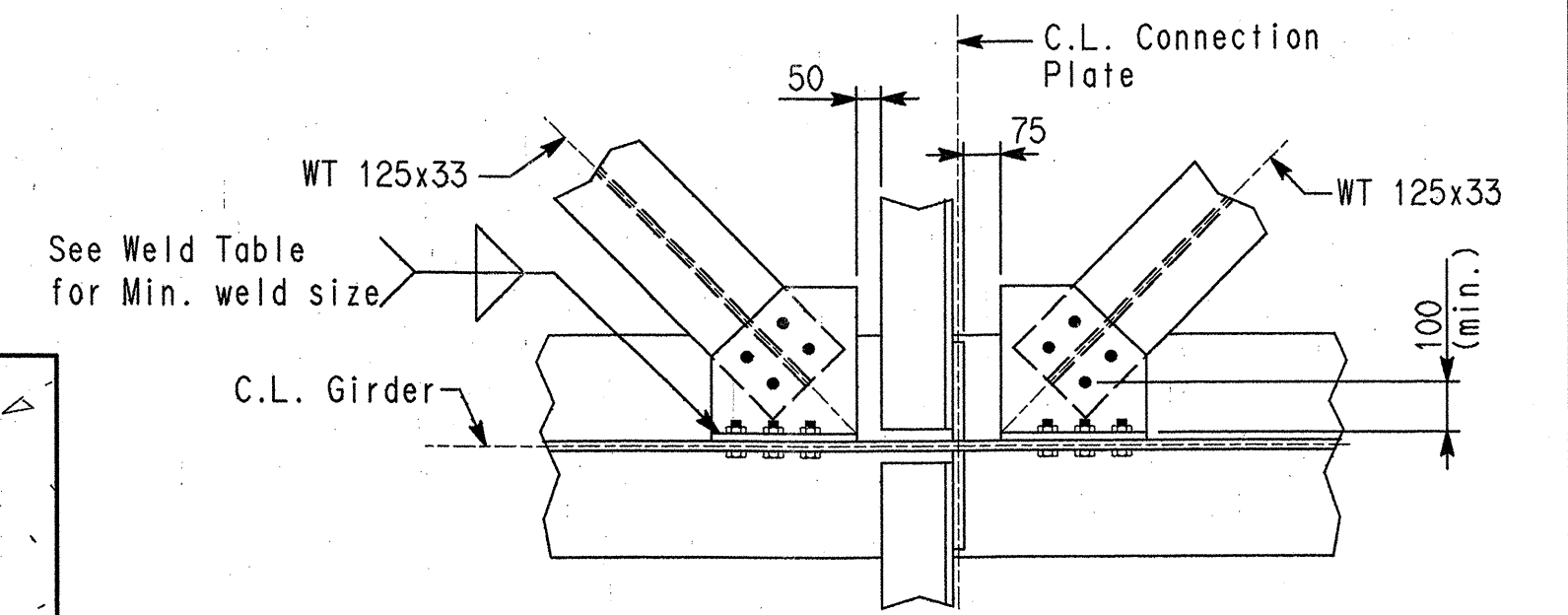
TYPICAL CROSS FRAME DETAIL
(Over Bents)



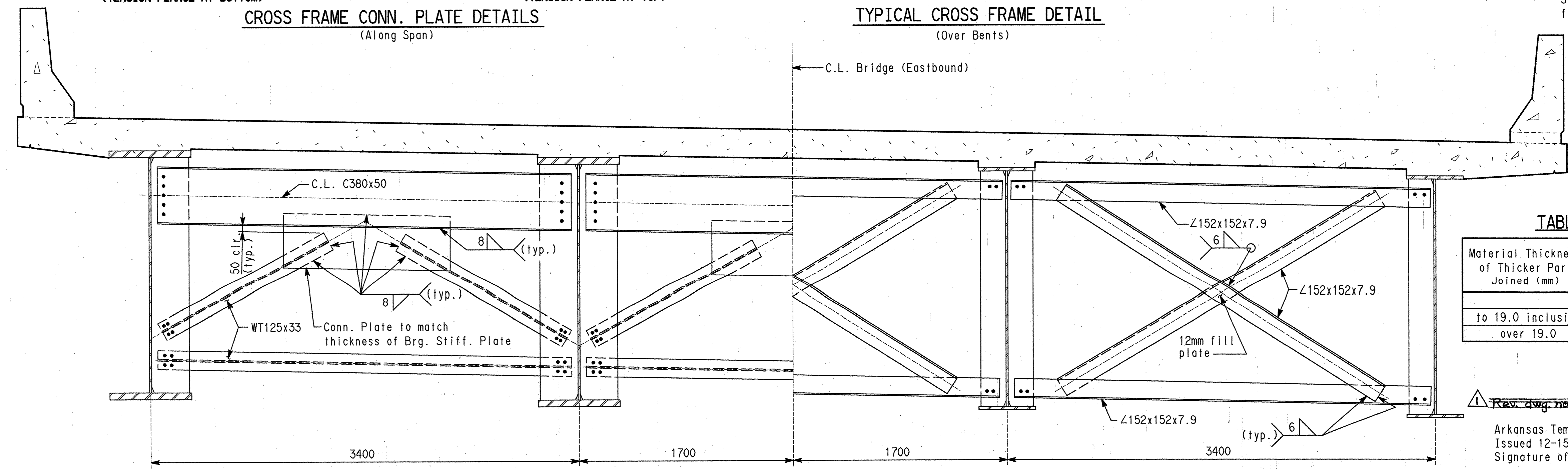
LATERAL BRACING
(Over Interior Bents)



LATERAL BRACING
(Between Cross Frames)



LATERAL BRACING
(Along Span)



TYPICAL OVER BENTS

CROSS FRAME SECTION
(Lateral Bracing not shown for clarity)

TYPICAL ALONG SPAN

TABLE FOR WELD

Material Thickness of Thicker Part Joined (mm)	Minimum Size of Fillet Weld (mm)	Single Pass Weld Must Be Used
to 19.0 inclusive	6	
over 19.0	8	

Rev. dwg. no. WRR 7-14-97

Arkansas Temporary Permit Number 96-48

Issued 12-15-96.

Signature of Holder *[Signature]*

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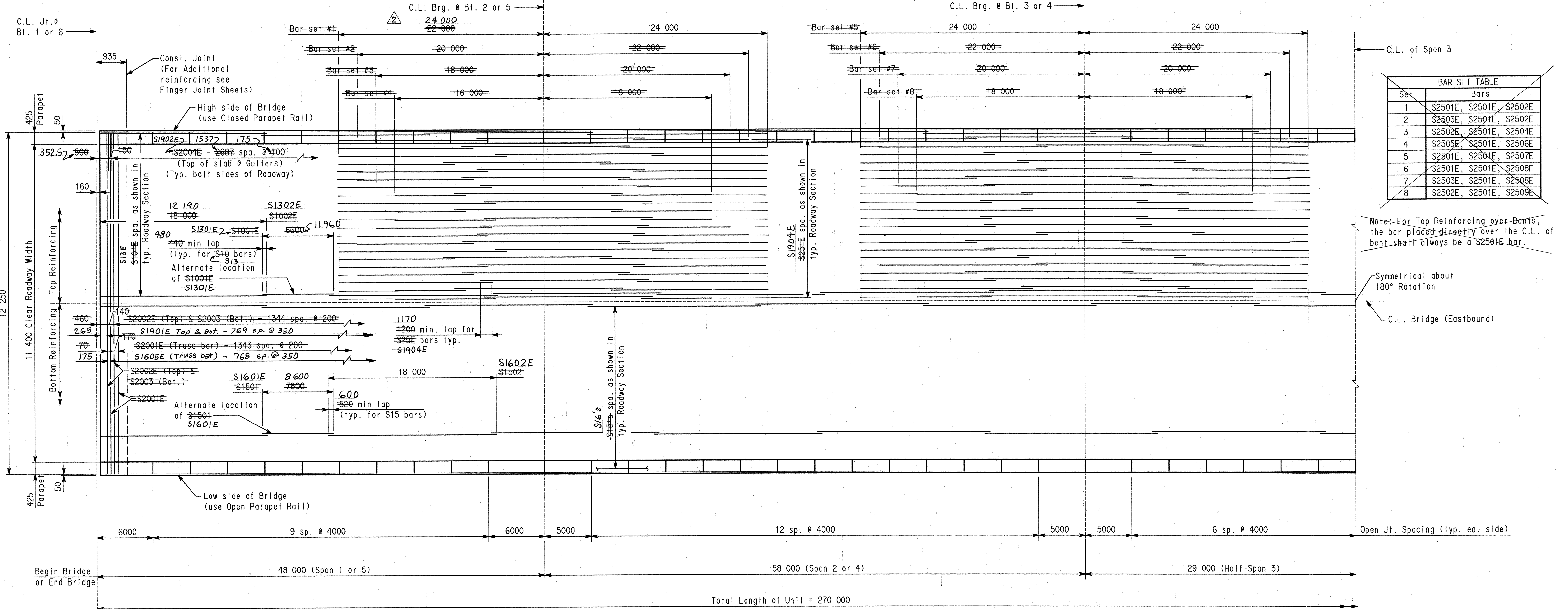
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EASTBOUND BRIDGE
SHEET 4 OF 5
270m CONTINUOUS PLATE GIRDER (CROSS FRAME)
US. HWY. 412
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARKANSAS

DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: No Scale
DESIGNED BY: MLR DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 38257 39257

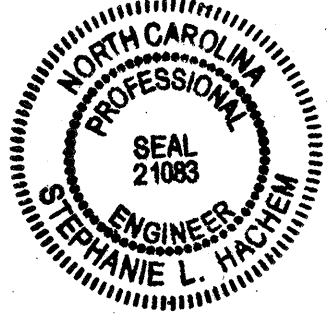
Note: For details of concrete parapet rail, see drwg. no. 38259 39259

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	Fed Road Dist. No.	State	Fed. and Proj No.	Sheet No.	Total Sheets
7/14/97 12-4-98	7/23/97 2-11-99				Ark.		34	118
				Job No.		040236		
				B6686		SLAB PLAN	38258	39258



SLAB PLAN

Scale Length 1:200, Width 1:100
(Parapet Reinforcing not shown for clarity)



Arkansas Temporary Permit Number 96-45,
Issued 12-15-96.
Signature of Holder *[Signature]* 4/1/97

SLAB JOINT DETAIL

No Scale

6 x 25 Type 6 Joint Sealer. See Sections 501.02 (h) and 501.05 (j) of the Standard Specification. Joint Sealer shall be measured and paid for as class S(AE) Concrete. If slab joints are to be sawed, they shall be sawed before any vehicular traffic is allowed on the unit.

1 Rev. dwg. no. WRR 7-14-97
2 Rev. Rein. Bars. 12-4-98

All dimensions are in millimeters (mm) unless otherwise noted.

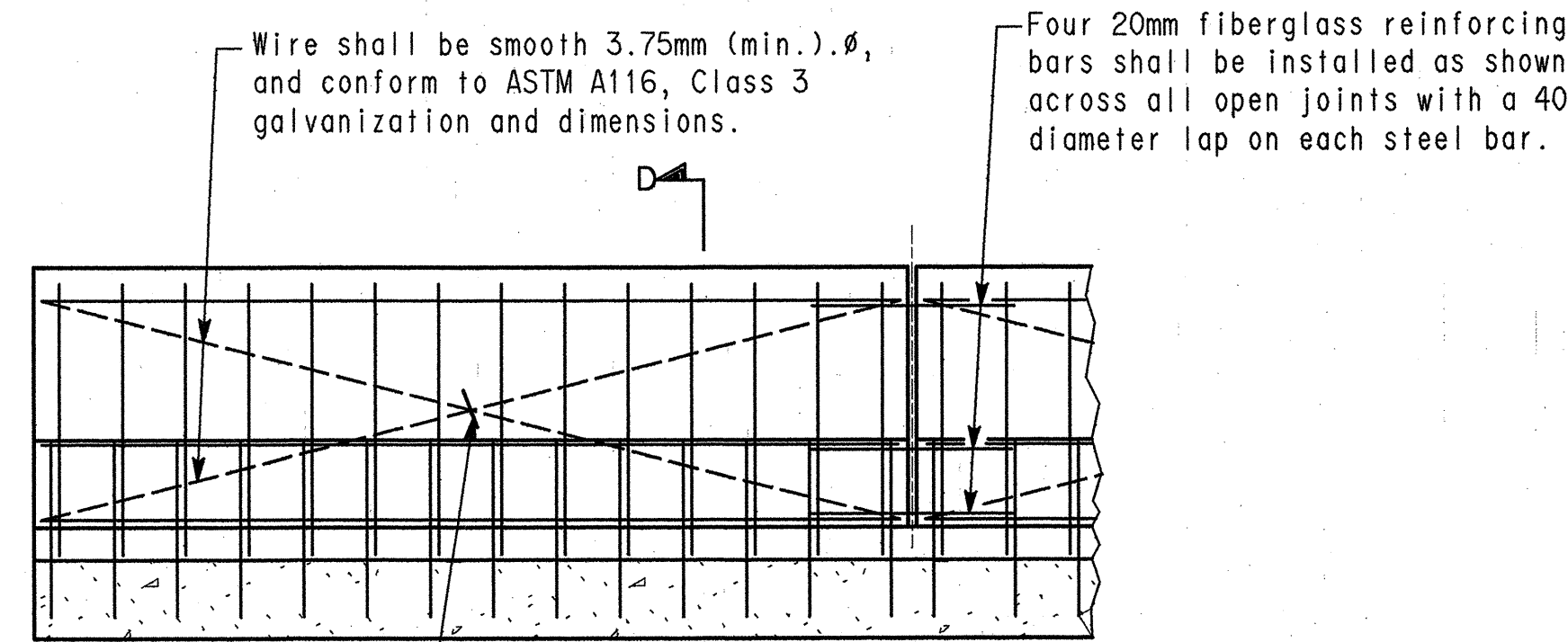


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EASTBOUND BRIDGE
SHEET 5 OF 5
270m CONTINUOUS PLATE
GIRDER (SLAB PLAN)
US. HWY. 412
ARKANSAS STATE HIGHWAY
COMMISSION
LITTLE ROCK, ARKANSAS

DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted
DESIGNED BY: SLH DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 38258 39258

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	Fed Road Dist. No.	State	Fed. and Proj No.	Sheet No.	Total Sheets
7-14-97	7/23/97				Ark.		35	118
12-4-98	2-11-99							
				Job No.		040236		
				B6686	PARAPET		30250	31259

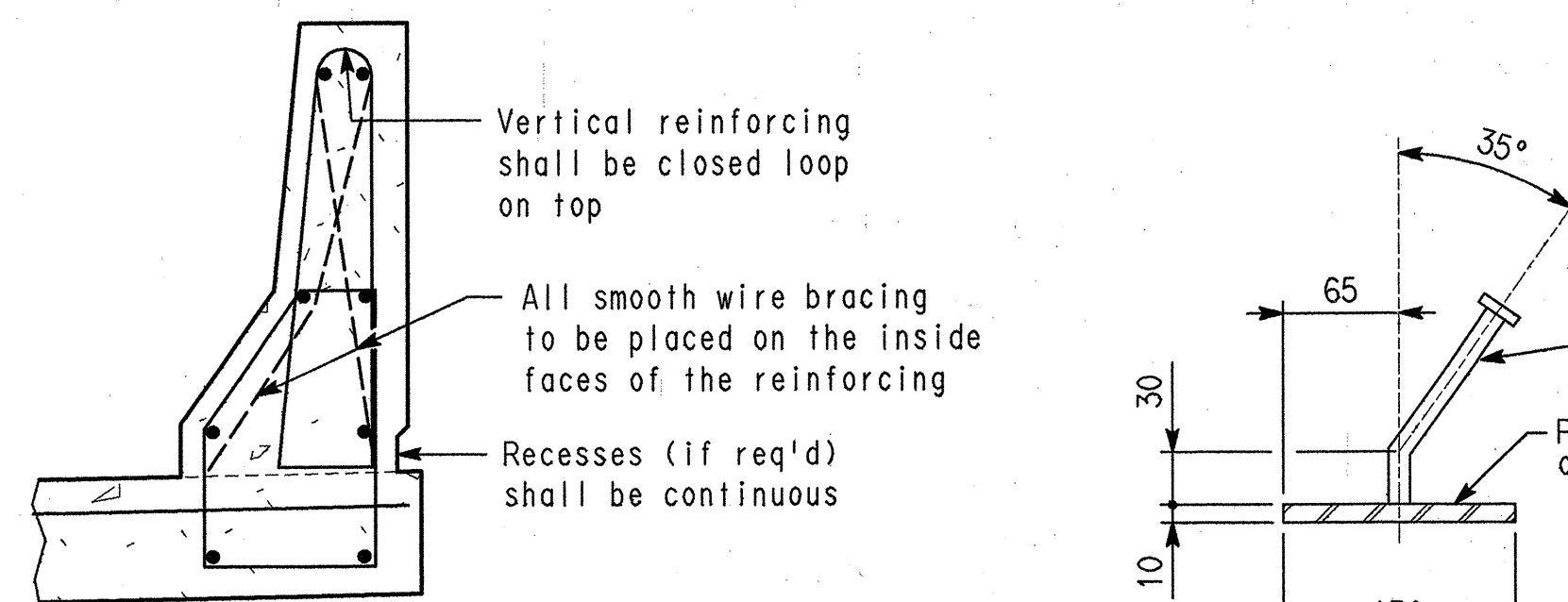


Bar to tighten smooth wire shall be epoxy coated of fiberglass.

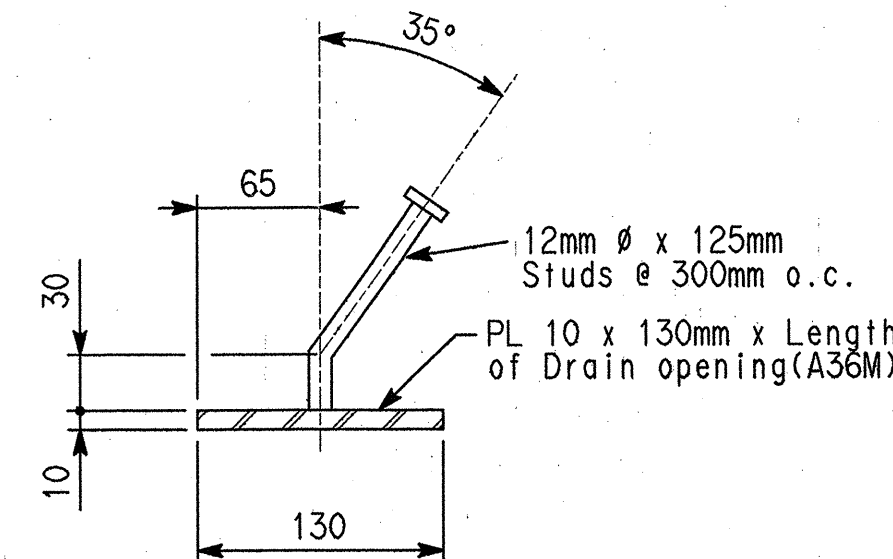
All panels shall be braced as shown to prevent racking. All open joints shall be sawed as soon as practical to a min. width of 6mm. to control cracking before sawing, all joints must be grooved before the concrete is set. Sawing of the joints must be controlled so it will follow the grooved joint.

DETAILS OF OPTIONAL SLIPFORMING OF CONCRETE PARAPET RAIL (OPEN OR CLOSED)

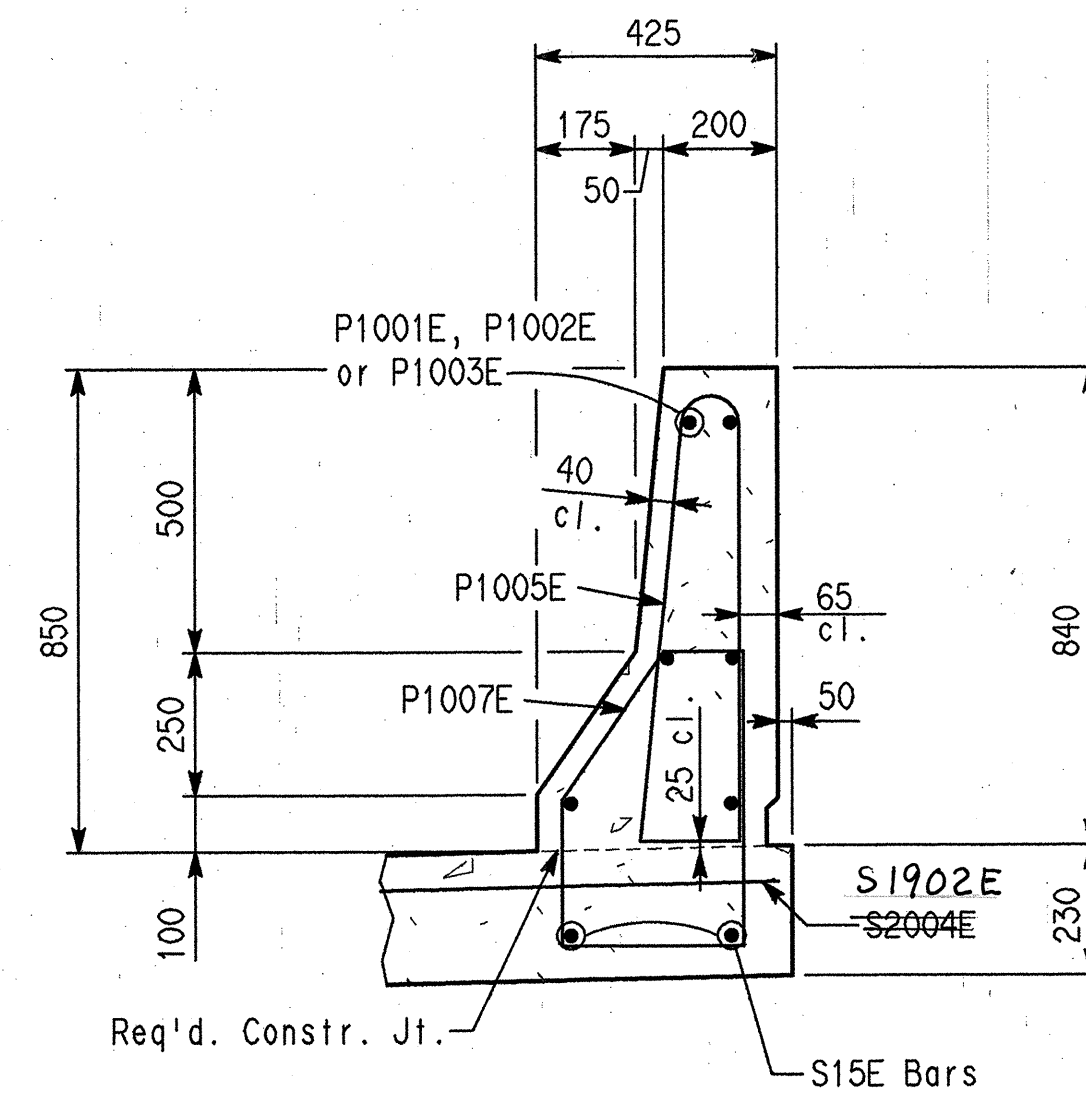
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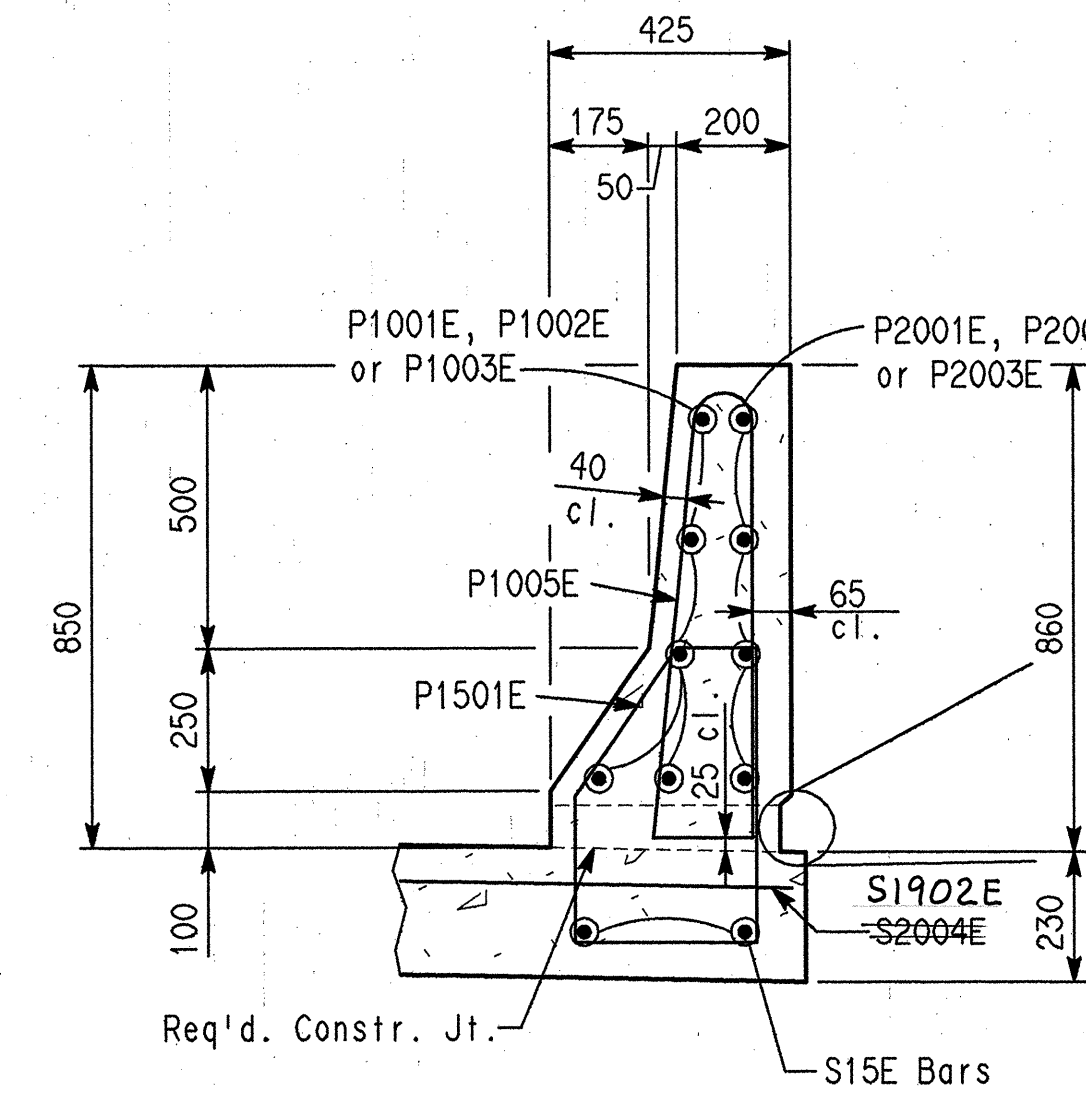
SECTION D-D
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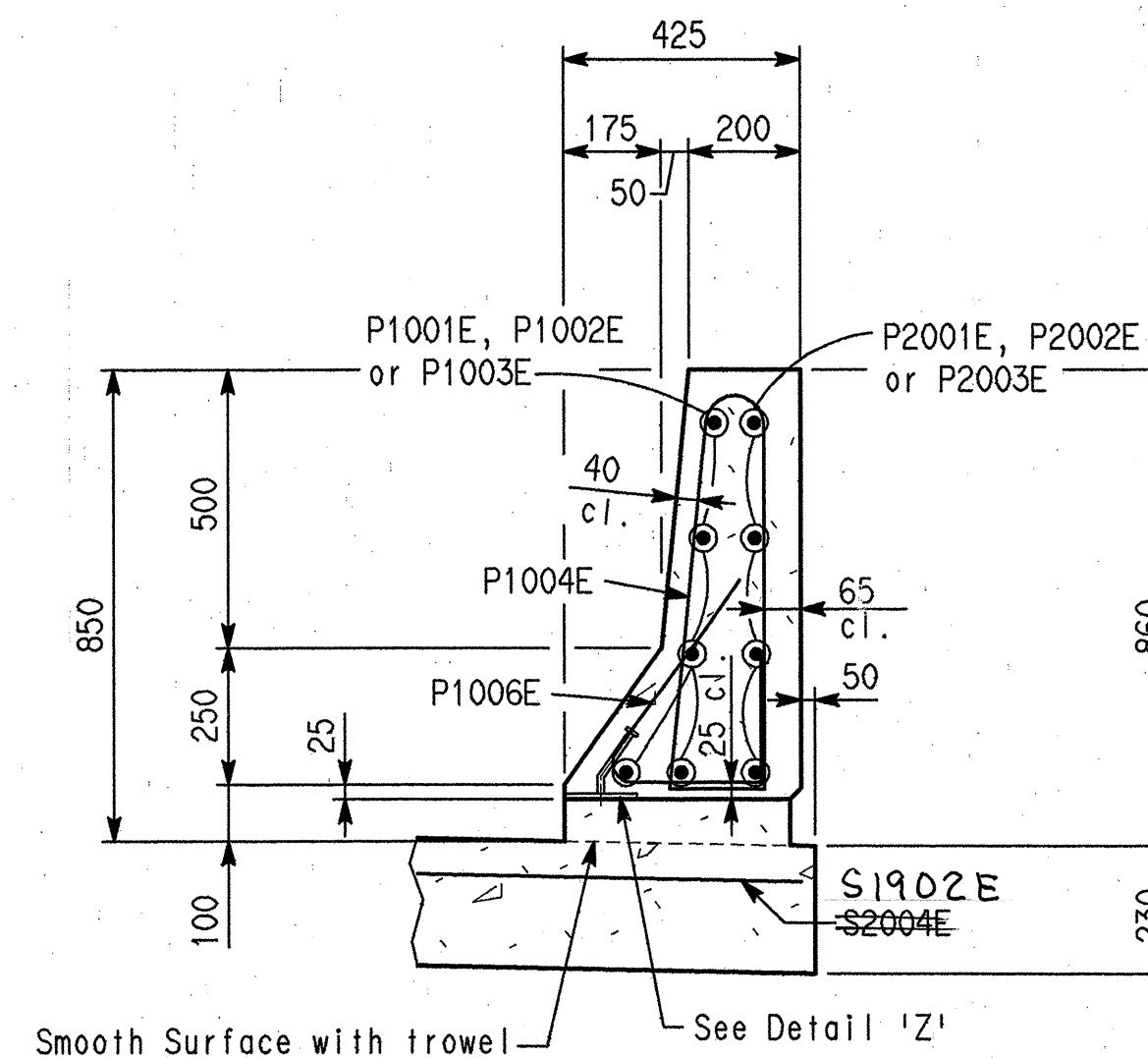
DETAIL Z
No Scale



SECTION A-A
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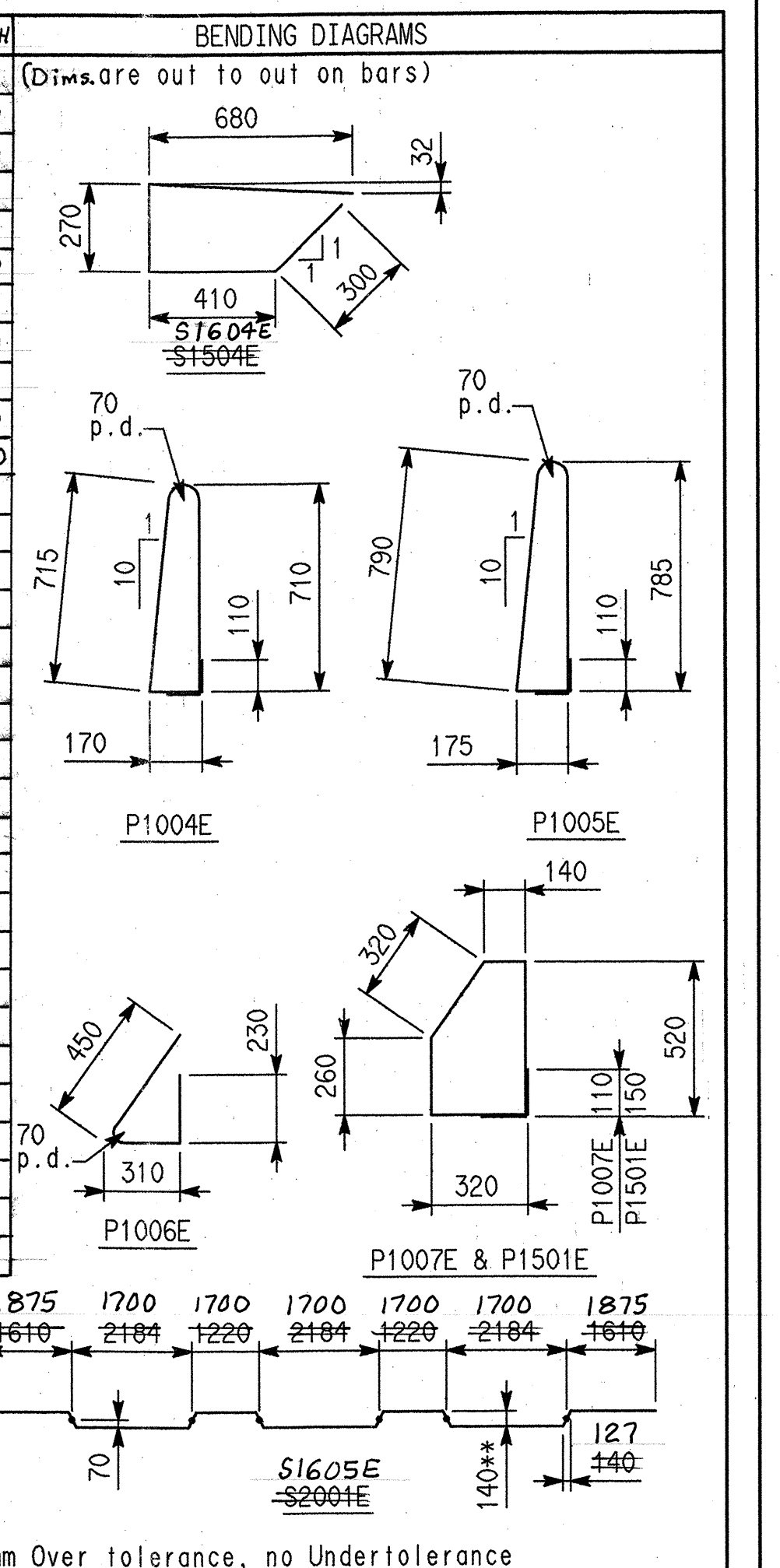


SECTION B-B
No Scale

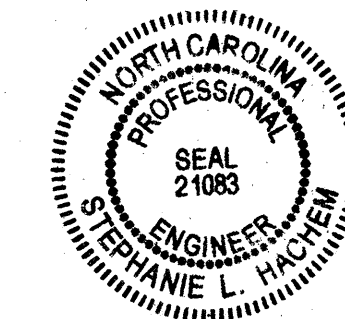


SECTION C-C
No Scale

MARK	MARK	NO.	REQ'D	LENGTH	P.D.	LENGTH
S1301E	S1001E	43	6600	Str.	11960	
S1302E	S1002E	645	946	Str.	12190	
S1601E	S1501	53	7800	Str.	8600	
S1602E	S1502	795	18000	Str.	18000	
S1603E	S1503E	18	3200	Str.	3200	
S1604E	S1504E	64	1600	70	1600	
S1605E	S2001E	1346	769	12	440	76120
S1901E	S2002E	1347	1540	12	140	12250
S2003E	S2003E	1347	12140	Str.		
S1902E	S2004E	3380	3076	1380	Str.	1720
S2201E	S2005E	8	12140	Str.	12250	
S2501E	S2501E	232	18000	Str.		
S2502E	S2502E	84	12400	Str.		
S2503E	S2503E	44	14000	Str.		
S2504E	S2504E	12	10000	Str.		
S2505E	S2505E	24	10800	Str.		
S2506E	S2506E	24	7600	Str.		
S2507E	S2507E	16	14400	Str.		
S2508E	S2508E	44	10400	Str.		
S2509E	S2509E	24	8000	Str.		
P1001E	P1001E	594	3900	Str.		
P1002E	P1002E	66	4900	Str.		
P1003E	P1003E	44	5900	Str.		
P1004E	P1004E	448	1800	50		
P1005E	P1005E	2590	1920	50		
P1006E	P1006E	448	990	50		
P1007E	P1007E	1350	1660	50		
P1501E	P1501E	1240	1690	70		
P2001E	P2001E	216	3900	Str.		
P2002E	P2002E	24	4900	Str.		
P2003E	P2003E	16	5900	Str.		
S1904E			720	16780	Str.	

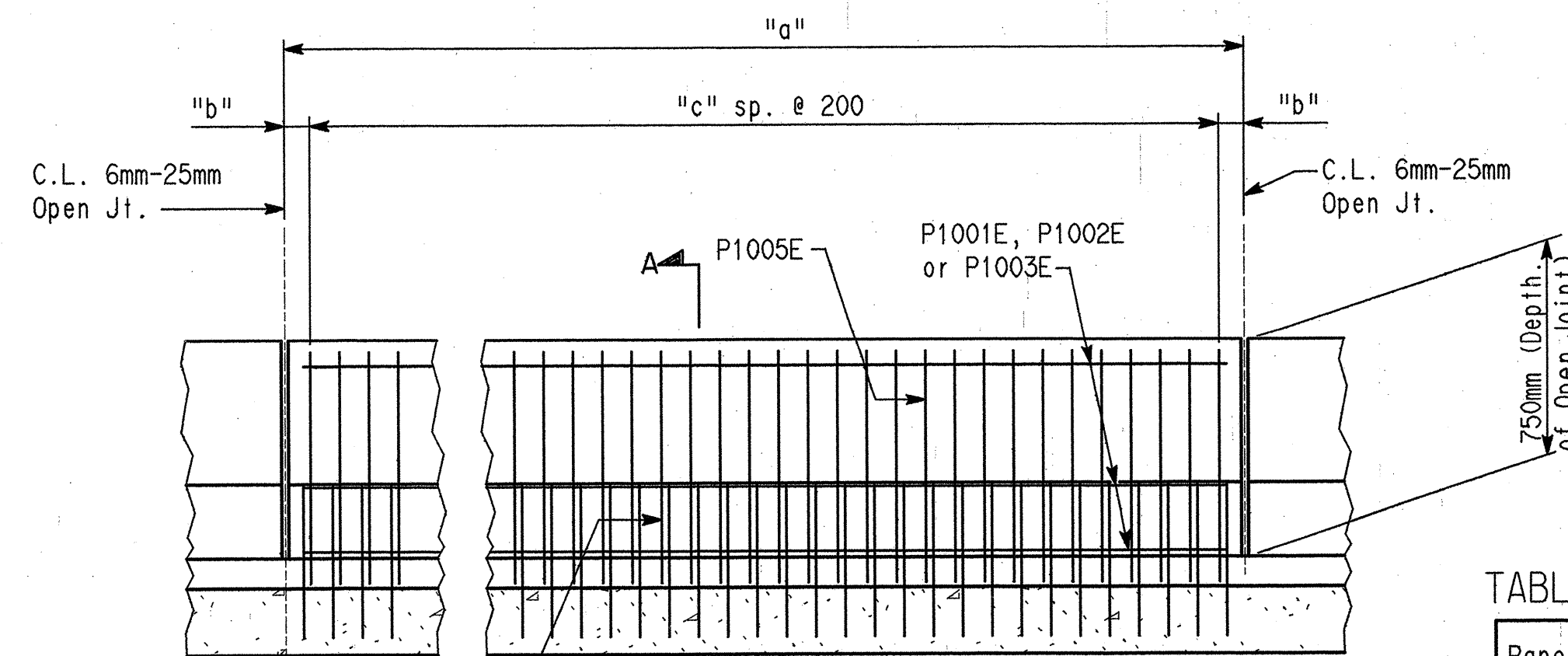


Note: The surfaces of the 10mm Plates which will not be in contact with concrete shall be painted in accordance with Section 638, or as approved by the Engineer. Only one coat is required and shall be applied in the fabricator's shop. Painting will not be paid for directly, but will be considered subsidiary to the item "Structural Steel in Beam Spans (AASHTO M270)."



Arkansas Temporary Permit Number 96-45, Issued 12-15-96. Signature of Holder *[Signature]* 4/12

All dimensions are in millimeters (mm) unless otherwise noted.

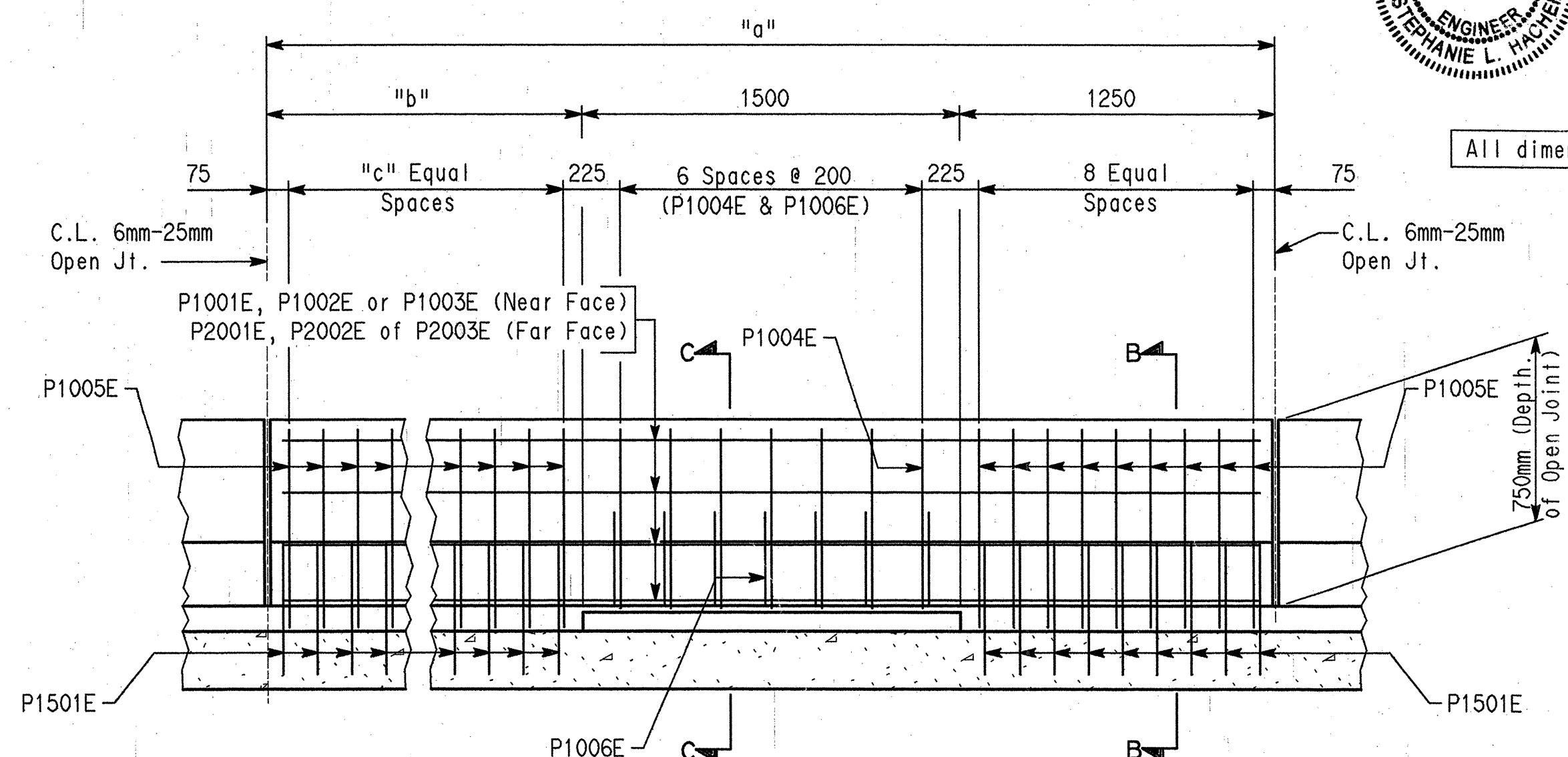


LONG. SECT. AT CURB FOR CLOSED PARAPET RAIL
No Scale

TABLE OF PARAPET RAIL VARIABLES

Panel Length "a"	Open Panel "b"	Open Panel "c"	Closed Panel "b"	Closed Panel "c"
4000	1250	8	100	19
5000	*2250	14	100	24
6000	*3250	21	100	29

* Parapet shall be constructed so that dimension "b" is started at the C.L. of Bent.



LONG. SECT. AT CURB FOR OPEN PARAPET RAIL
No Scale

Rev. dwg. no. WRR 7-14-97
Rev. Reinf. Bars. LDF 12-4-98



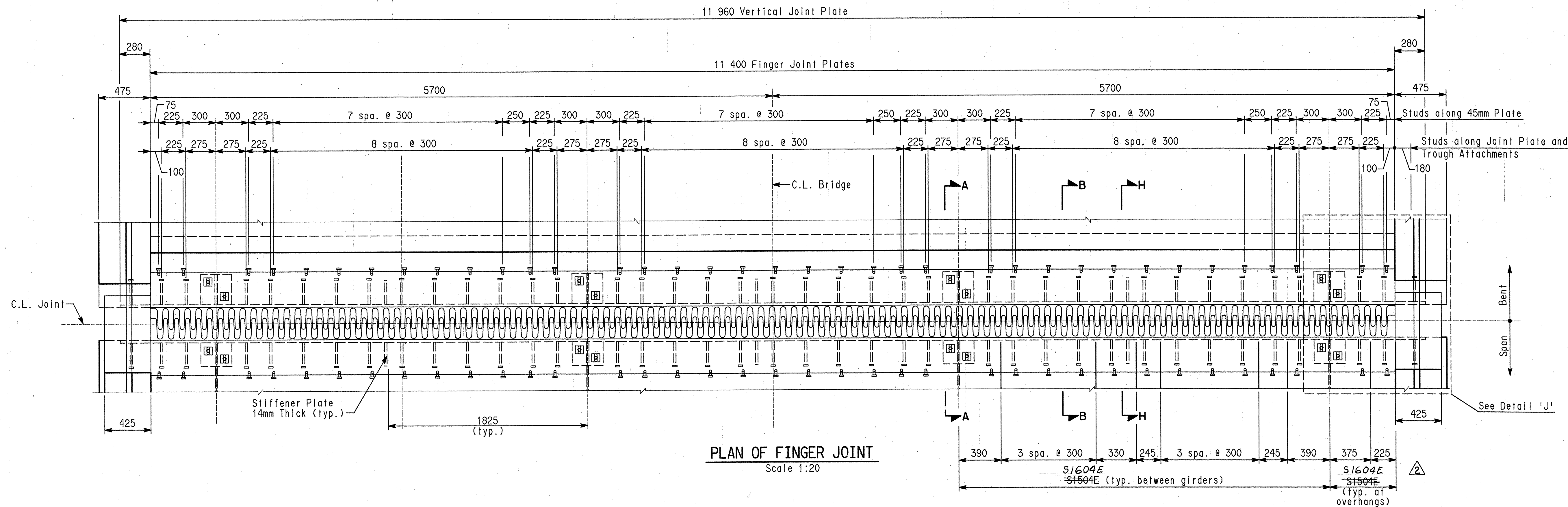
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EASTBOUND BRIDGE NOTES AND PARAPET DETAILS

US. HWY. 412, ARKANSAS STATE HIGHWAY COMMISSION, LITTLE ROCK, ARKANSAS

DRAWN BY: TBI, DATE: 12/96, CHECKED BY: SLH, DATE: 12/96, SCALE: As Noted, DESIGNED BY: CLN, DATE: 12/96, BRIDGE NO. B6686, DRAWING NO. 30250 31259

DATE REVISED	DATE FILMED	DATE REVISED	DATE FILMED	Fed Road Dist. No.	State	Fed. and Proj No.	Sheet No.	Total Sheets
7-14-97	7/23/97				Ark.		36	118
12-4-98	2/11/99					040236		
				Job No.				
				B6686		FINGER JOINT		30260 39260



NOTES

Unless otherwise noted, structural steel shall be AASHTO M270, Grade 345W and shall be fabricated in accordance with Section 807 of the Standard Specifications for Highway Construction.

Bolts, nuts and washers in trough shall be ASTM A278, type 304 stainless steel.

Material for troughs shall be nylon reinforced neoprene 6mm thick sheet.

Finger plates shall be flame cut from one plate by a single cut of a machine guided torch. Sharp corners are to be removed by grinding. Width of cut shall be 6mm.

Joint shall be fabricated to follow the grade and the transverse contour of the roadway.

To assure that all bedding areas and recesses of the structural elements are completely filled with well compacted concrete, adequate venting, vibrating and hand packing of concrete into these areas shall be done.

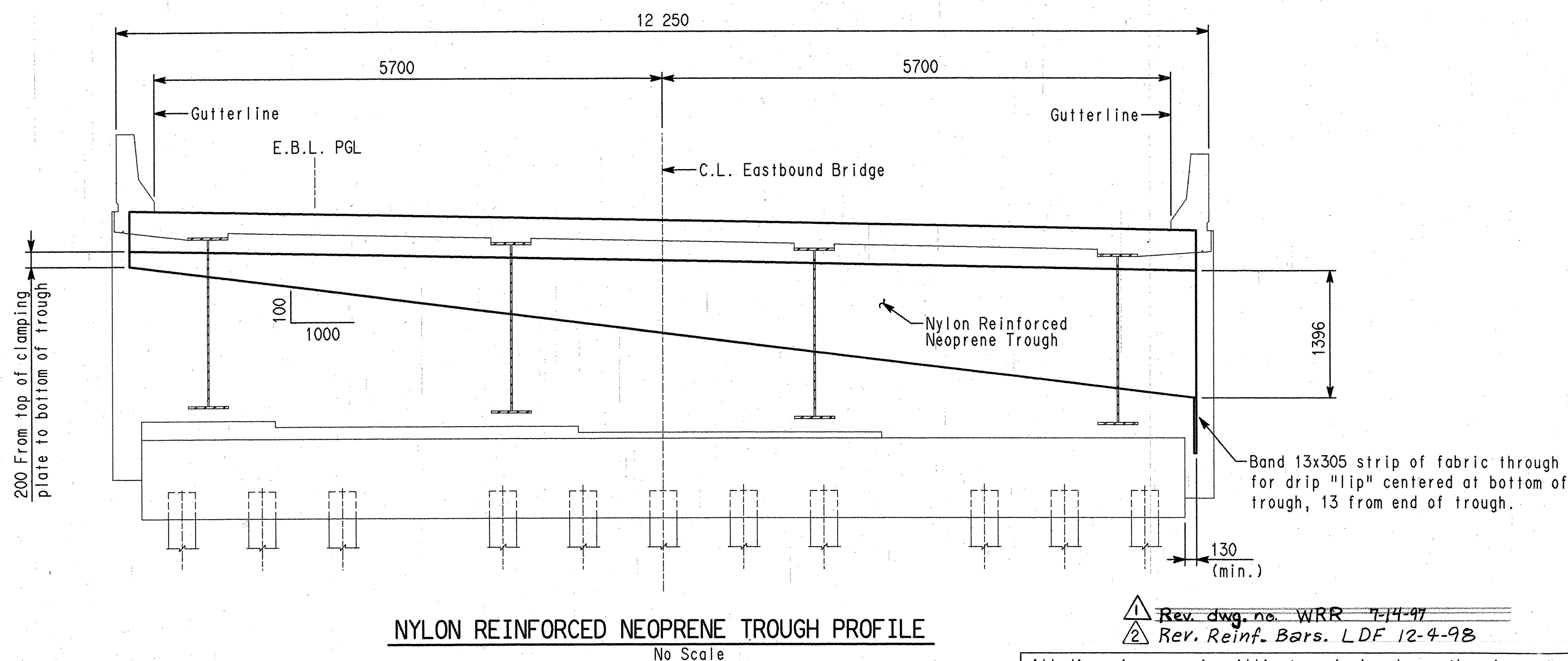
The studs shall be granular flux filled, solid fluxed, or equal and automatically end welded to the plates in accordance with the recommendations of the manufacturer.

Anchor bolts shall be cast in the position shown when the concrete is placed in the backwall.

Units shall be shipped to the job site preassembled.

Temporary L102 x 76 x 12.7 at maximum 1.5m centers shall be shop welded. After erection and adjustment, bolts shall be tightened. After concrete has been set, angles shall be removed by chipping connection welds and grinding surfaces smooth.

Set Joint and place blockout concrete after the deck slab in the adjacent span has been placed. Before placing blockout concrete, apply epoxy bonding agent to transverse construction joint.



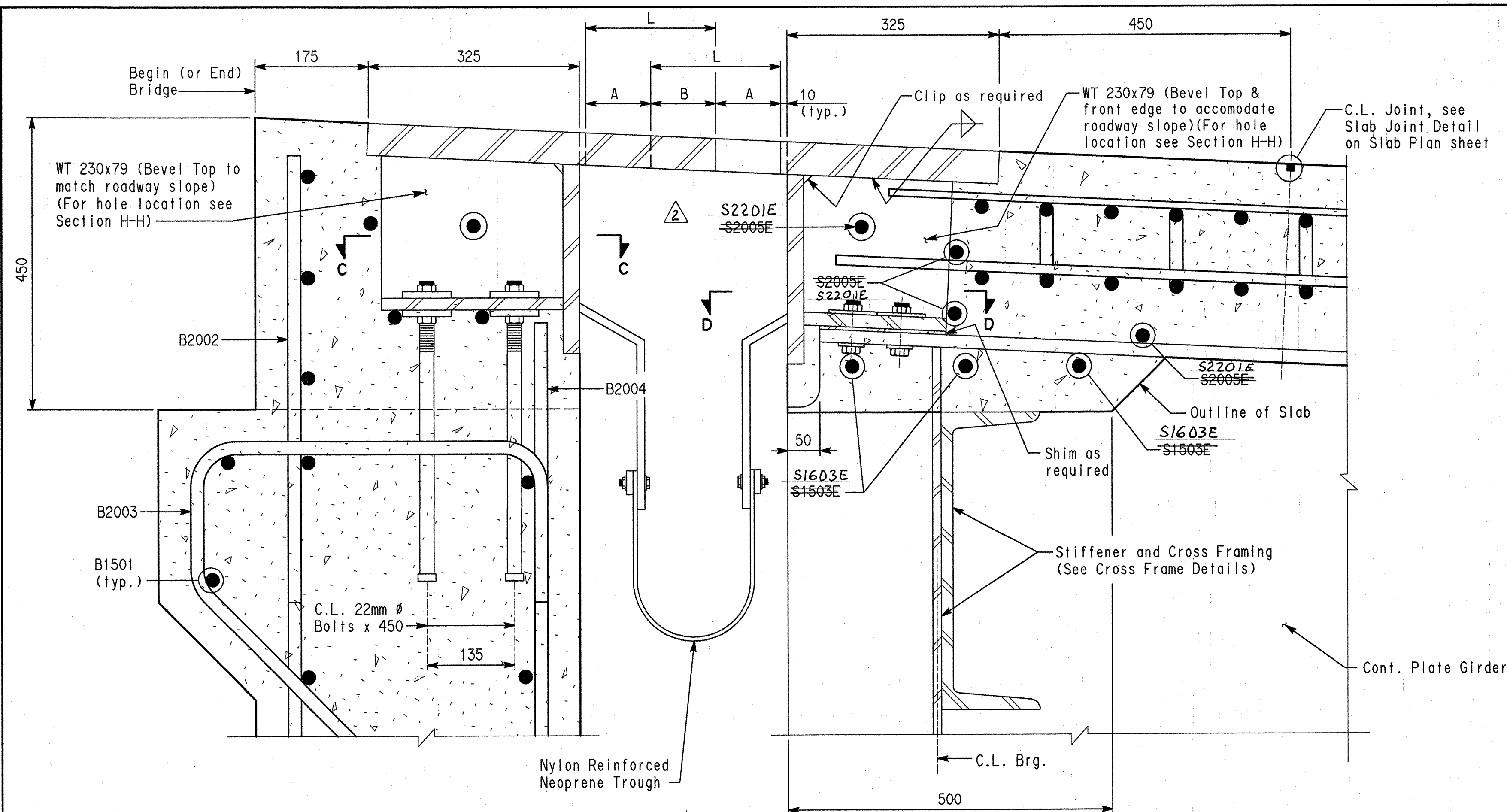
Arkansas Temporary Permit Number 96-45,
Issued 12-15-96.
Signature of Holder *SLH* 2/7/97

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**EASTBOUND BRIDGE
SHEET 1 OF 3
FINGER JOINT
(PLAN & PROFILE)**
US. HWY. 412
**ARKANSAS STATE HIGHWAY
COMMISSION**
LITTLE ROCK, ARKANSAS
DRAWN NO. TBI DATE: 12/96
CHECKED BY: CLN DATE: 12/96 SCALE: As Noted
DESIGNED BY: SLH DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 30260 39260

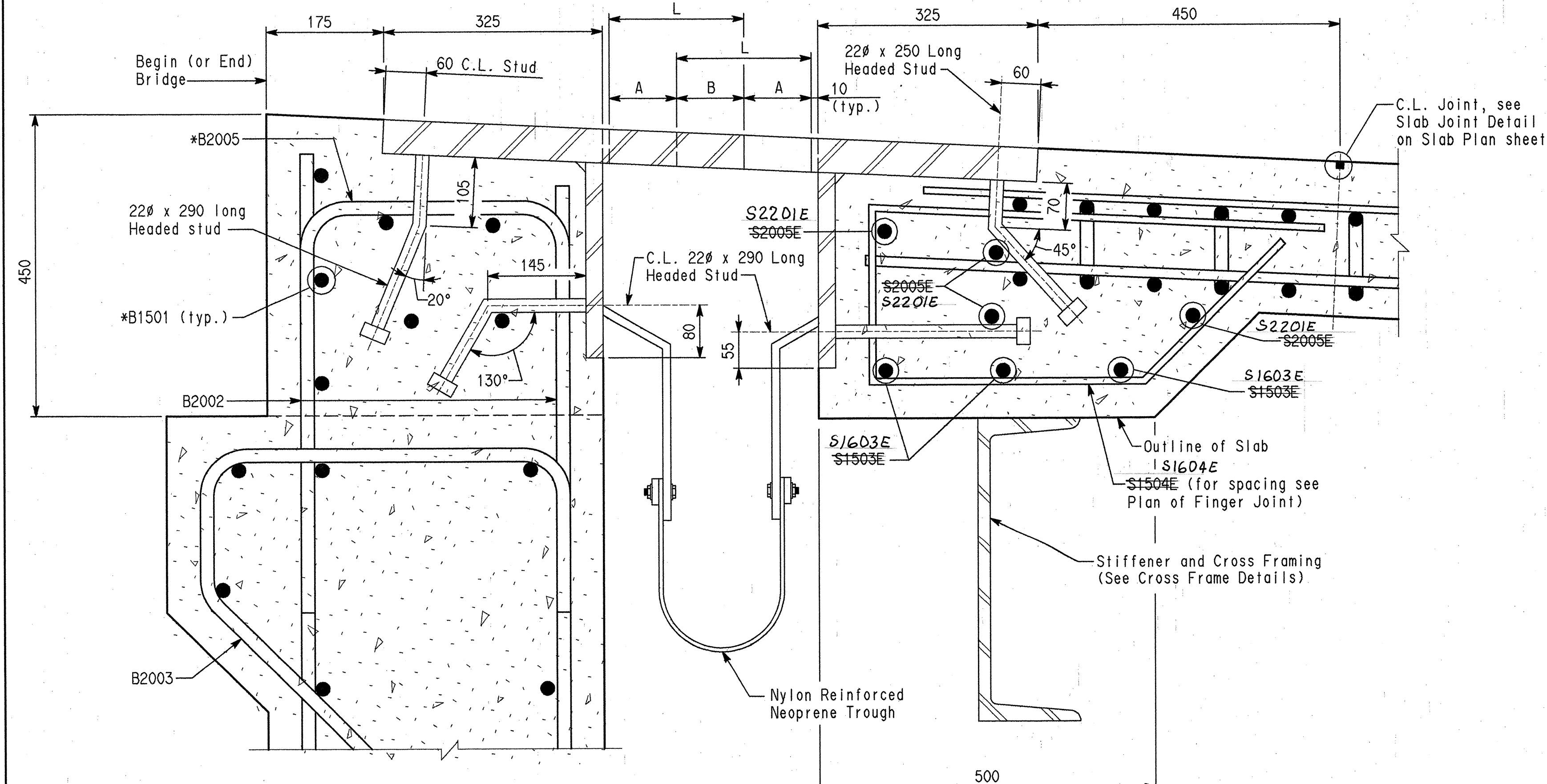
Rev. dwg. no. WRR 7-14-97
Rev. Reinf. Bars. LDF 12-4-98
All dimensions are in millimeters (mm) unless otherwise noted.

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				Job No.		040236		
				B6686		FINGER JOINT	38261	39261

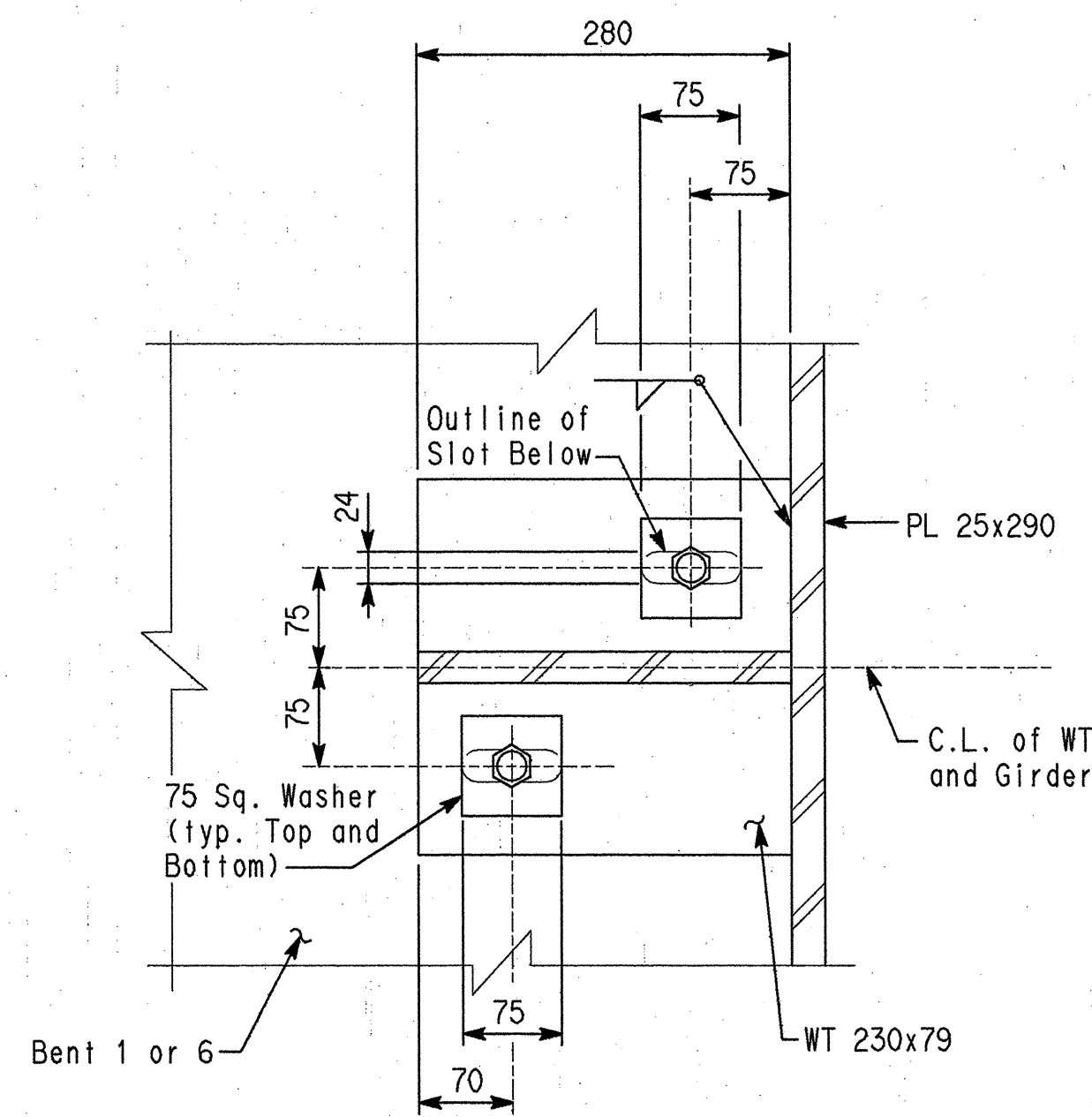


* See Bent 1 or 6 for Bar List.

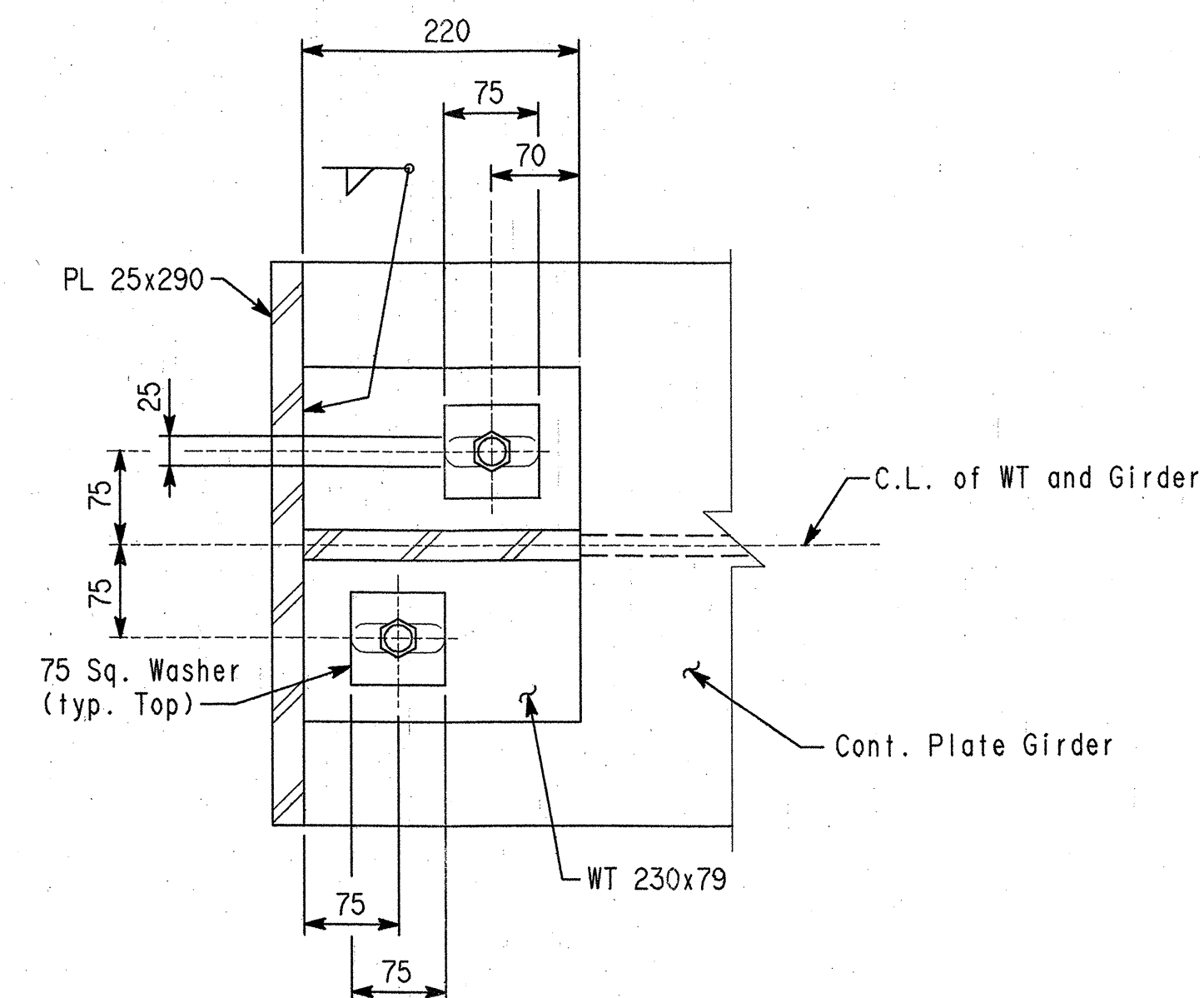
SECTION A-A
For End Bent Reinforcing (See Bent 1 or 6)
See Slab Plan for Bar Spacing



SECTION B-B
For End Bent Reinforcing (See Bent 1 or 6)
See Slab Plan for Bar Spacing

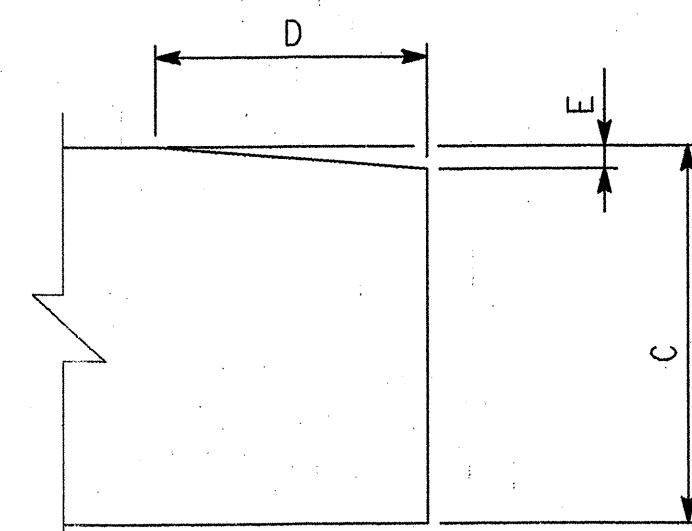


SECTION C-C



SECTION D-D

A	B	C	D	E	L
95	110	45	40	3	205



FINGER BEVEL DETAIL

Rev. Reinf. Bars. LDF 12-4-98
Rev. dwg. no. WRR 7-14-97



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EASTBOUND BRIDGE
SHEET 2 OF 3
FINGER JOINT
(DETAILS)
US. HWY. 412
ARKANSAS STATE HIGHWAY
COMMISSION
LITTLE ROCK, ARKANSAS

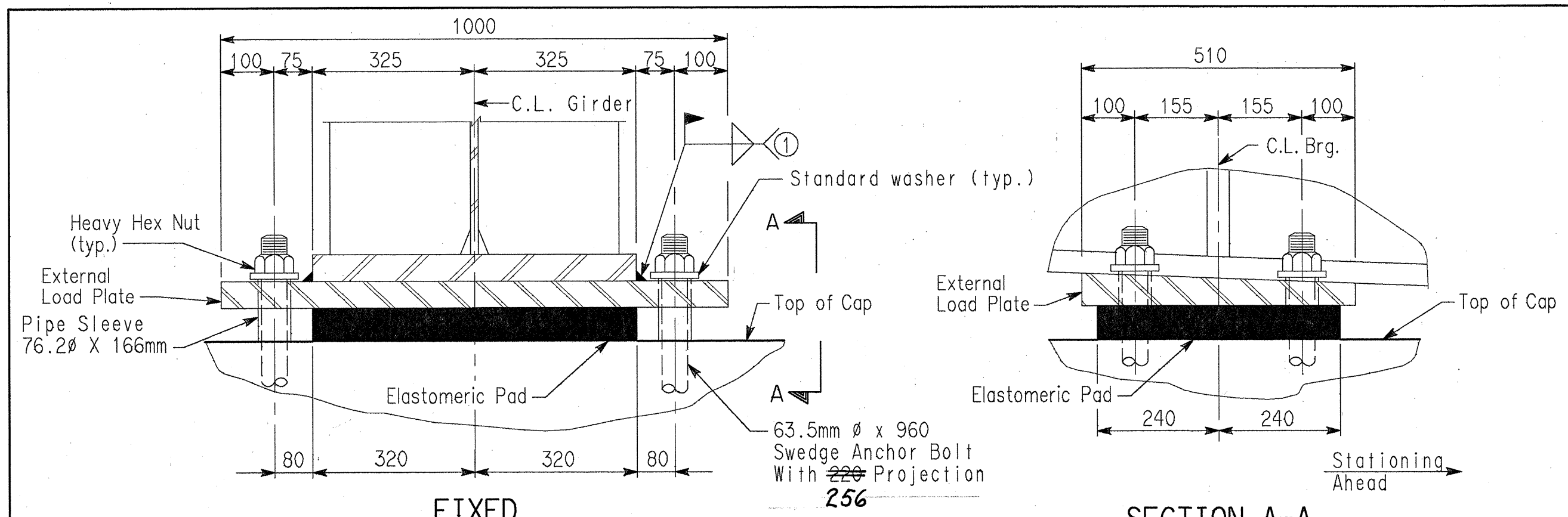


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Signature of Holder *SLH* 7/2/97

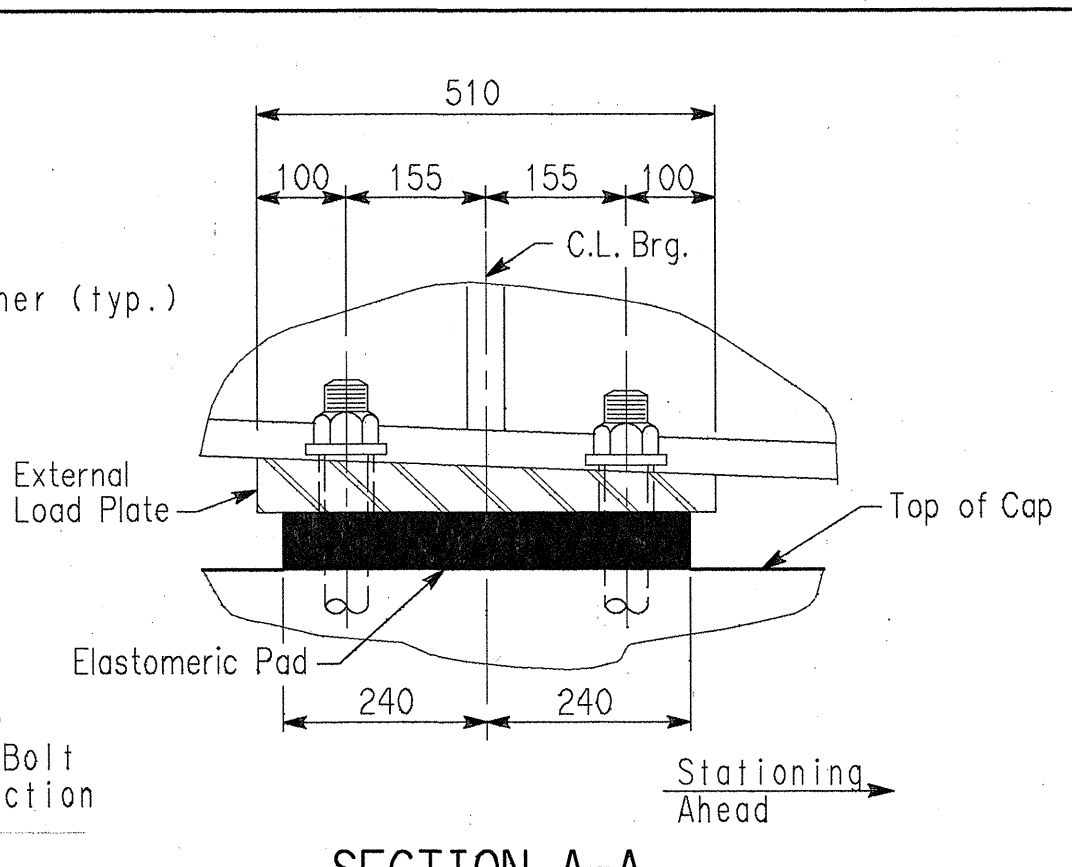
All dimensions are in millimeters (mm) unless otherwise noted.

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CHECKED BY: CLN DATE: 12/96 SCALE: 1:5
DESIGNED BY: SLH DATE: 12/96
BRIDGE NO. B6686 DRAWING NO. 38261 39261

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7-14-97	7-24-97							
10-23-97	10-24-97							
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				B6686		BEARING PADS	39263	39263

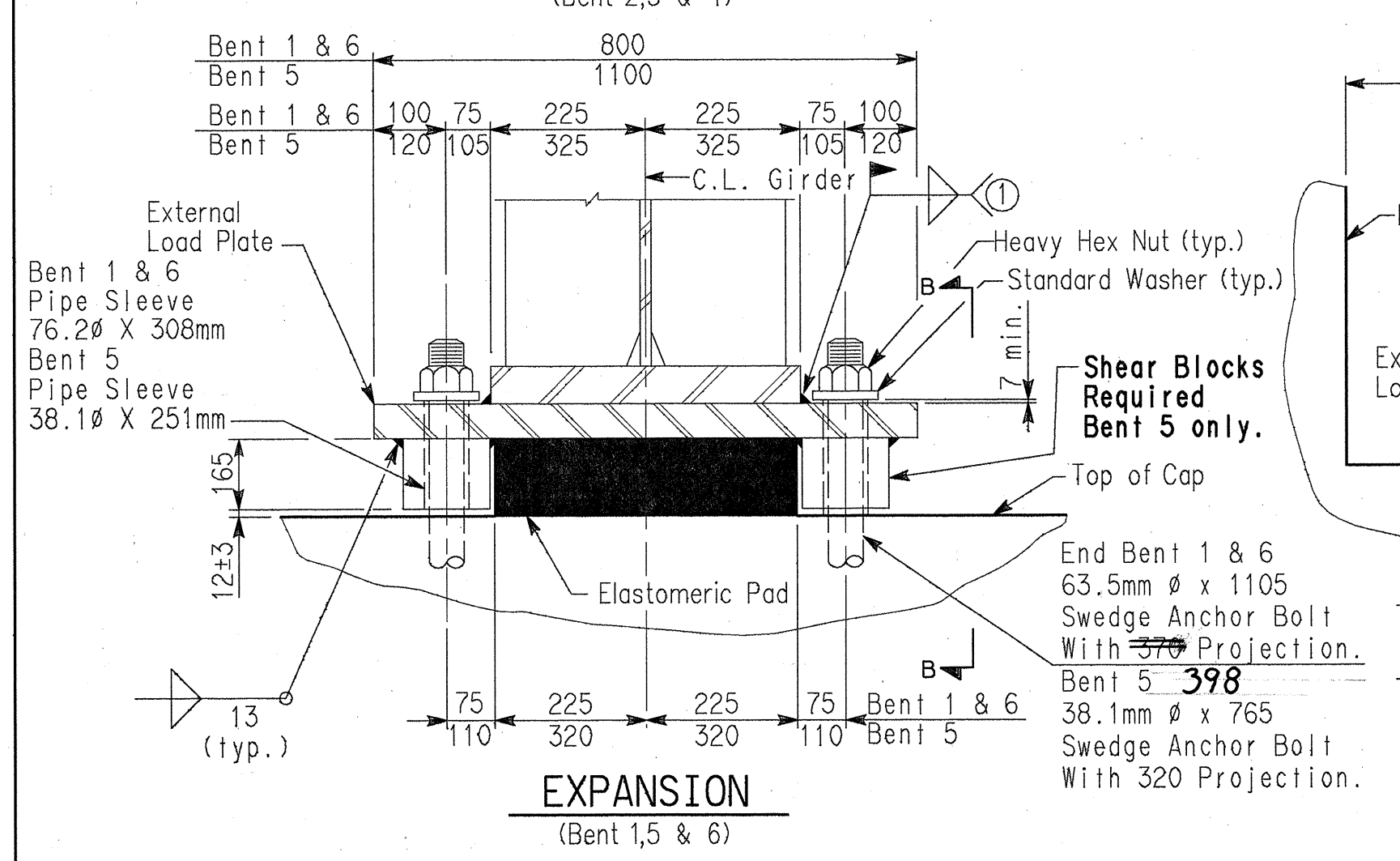


FIXED
(Bent 2,3 & 4)

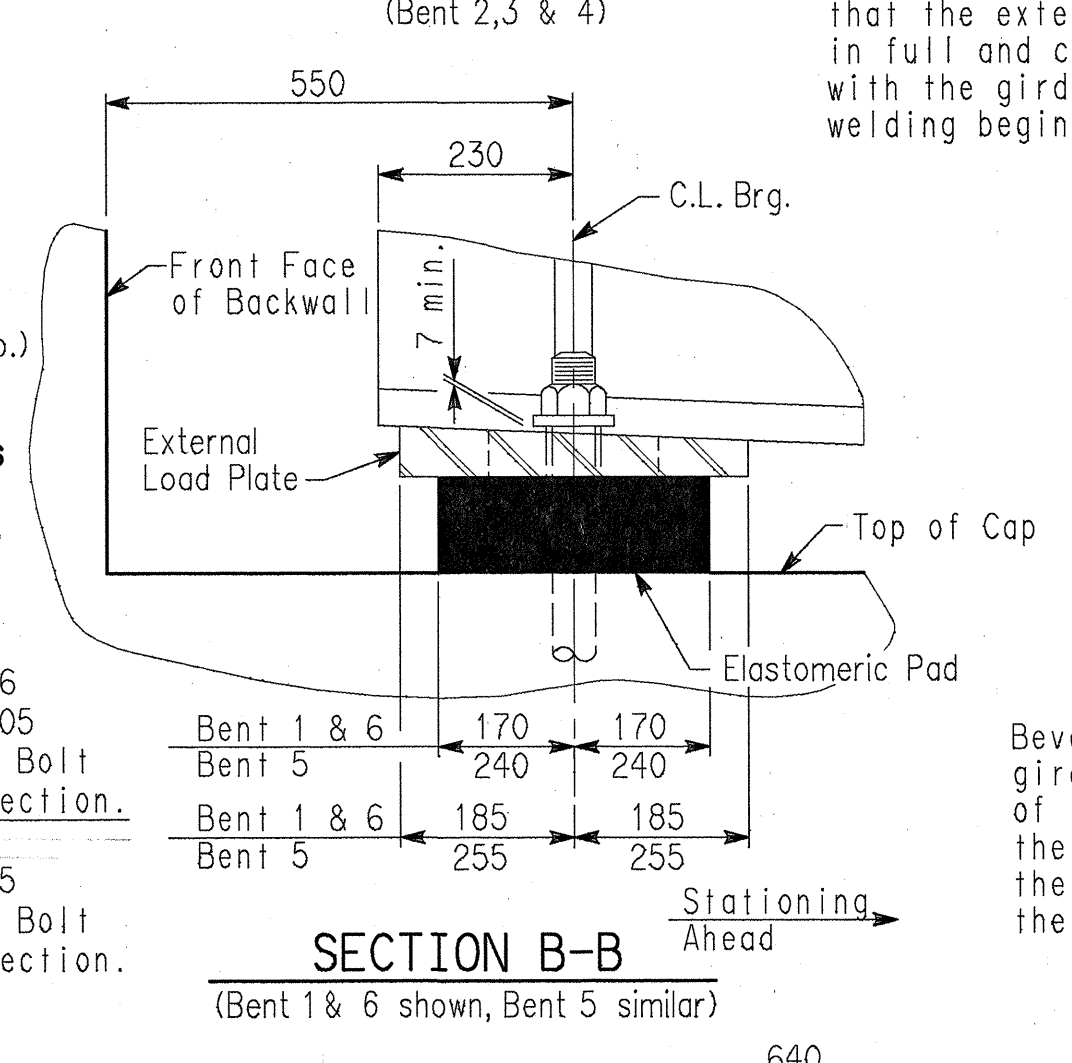


SECTION A-A
(Bent 2,3 & 4)

① Care shall be taken to ensure that the external load plate is in full and complete contact with the girder flange before welding begins.

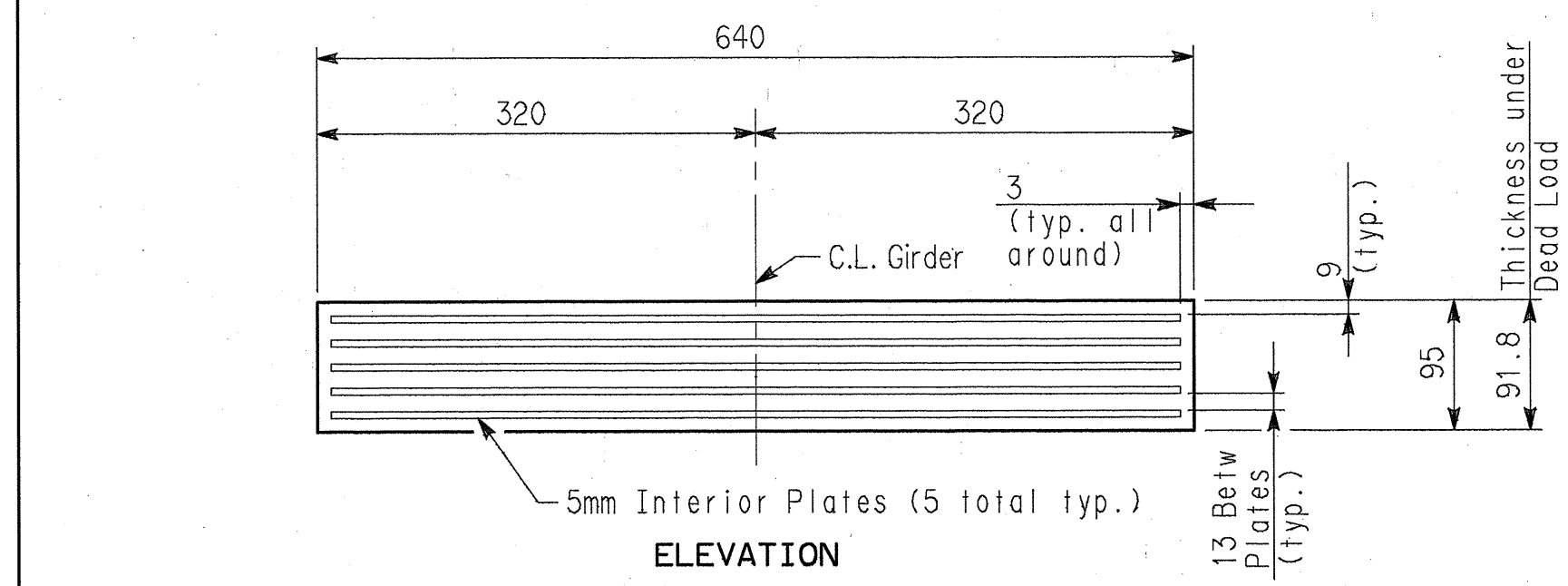


EXPANSION
(Bent 1,5 & 6)

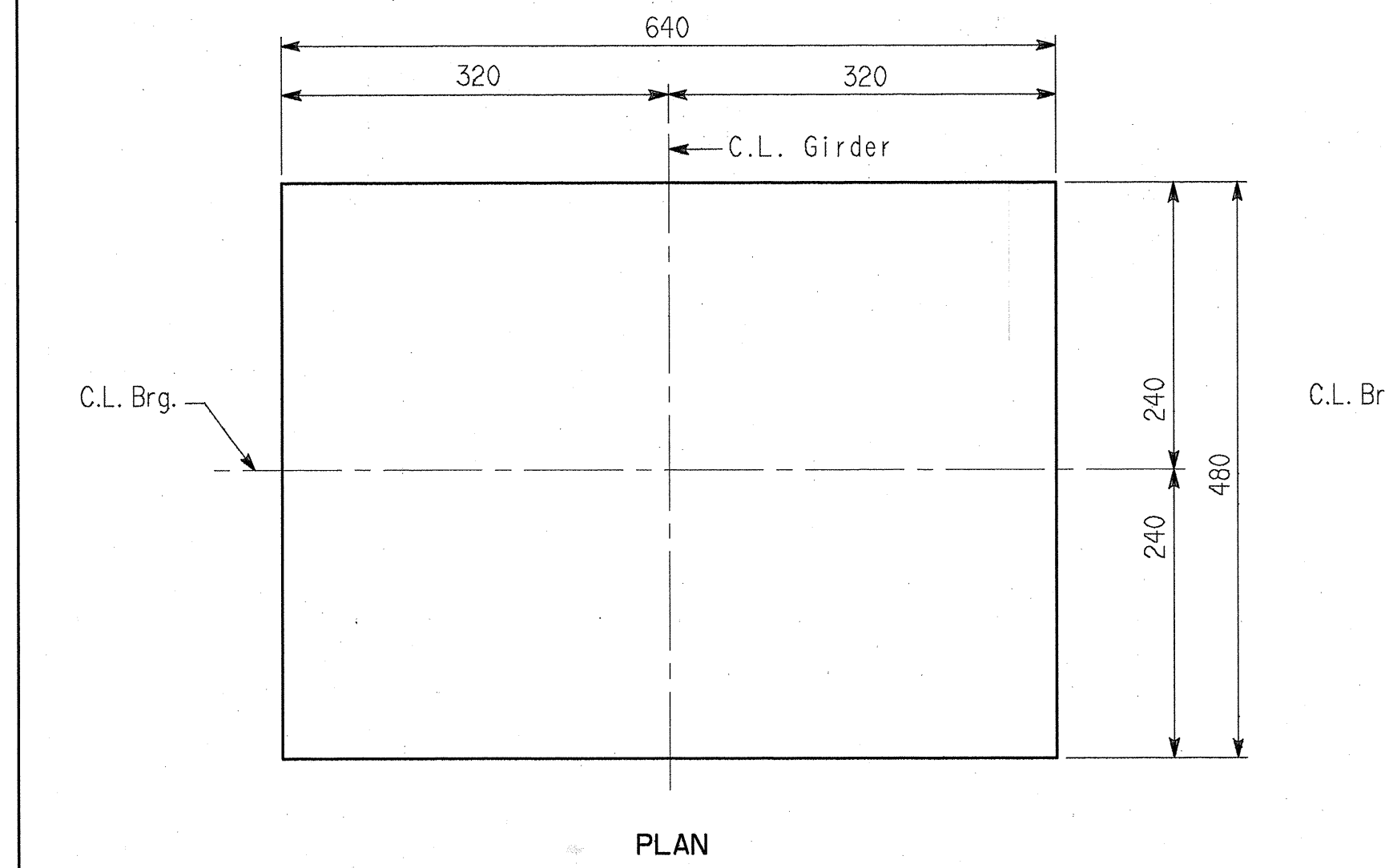


SECTION B-B
(Bent 1 & 6 shown, Bent 5 similar)

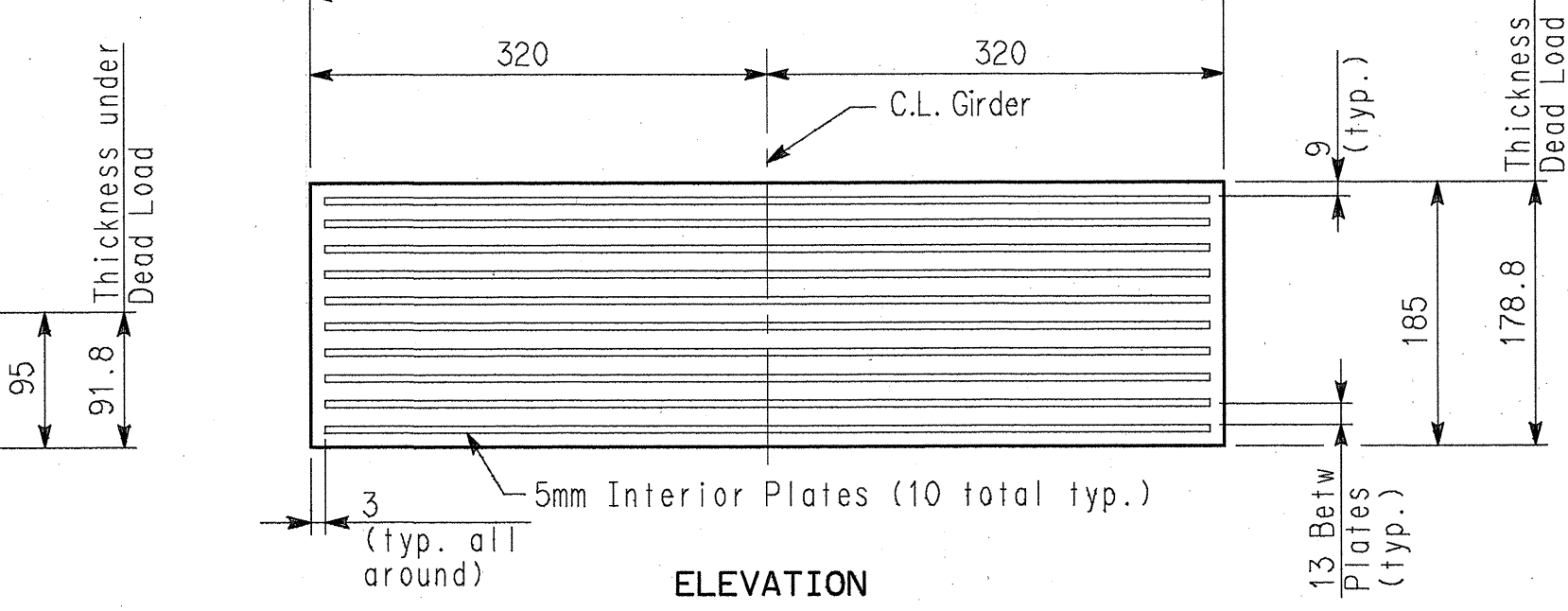
Bevel External Load Plate to match slope of girder after dead load deflection. The location of the Anchor Bolts in relation to the holes in the External Load Plate shall correspond with the temperature at the time of erection. At 15° C the holes should center on the anchor bolts.



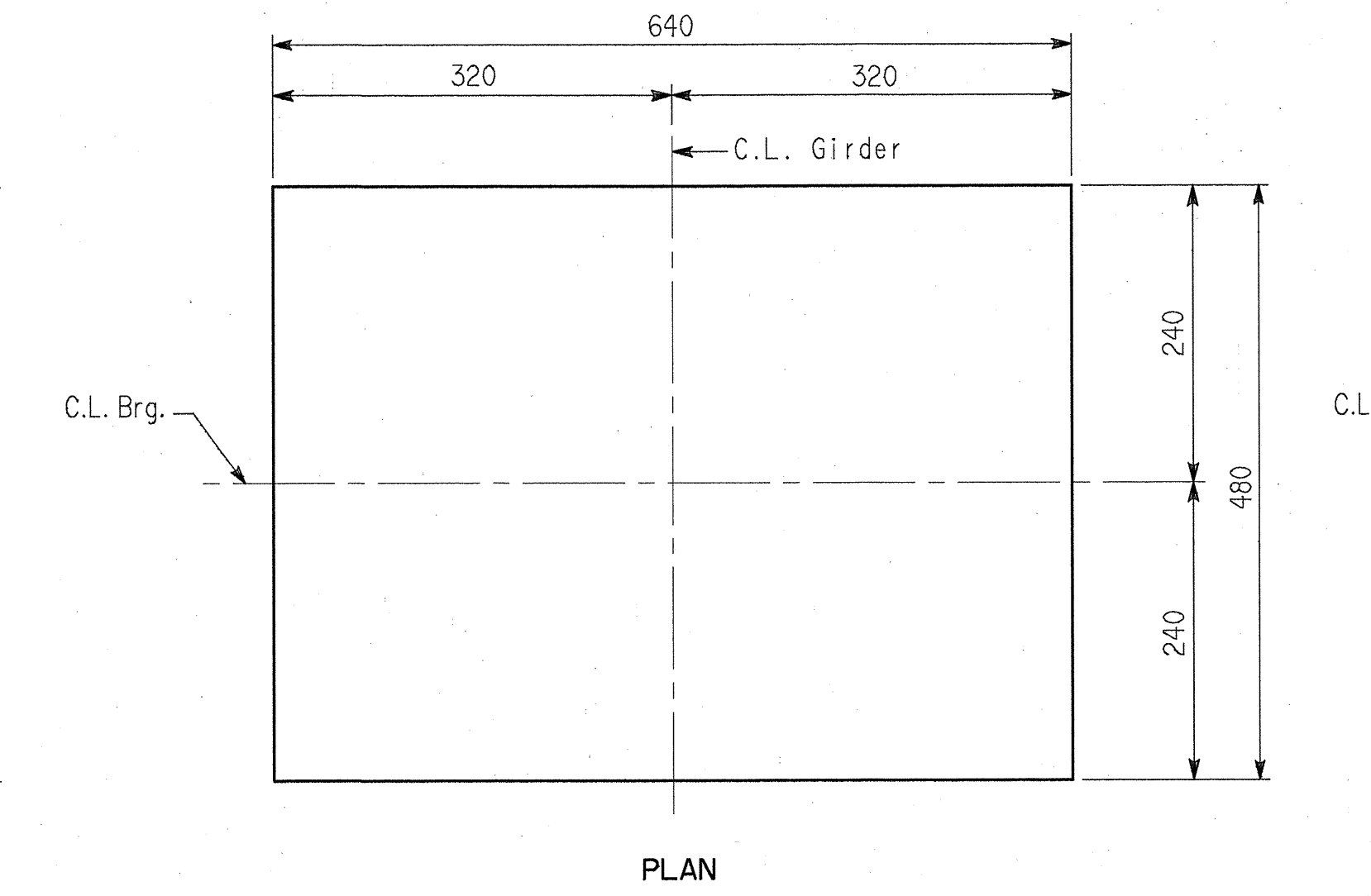
ELASTOMERIC BEARING PAD
(Bent 2,3 & 4)



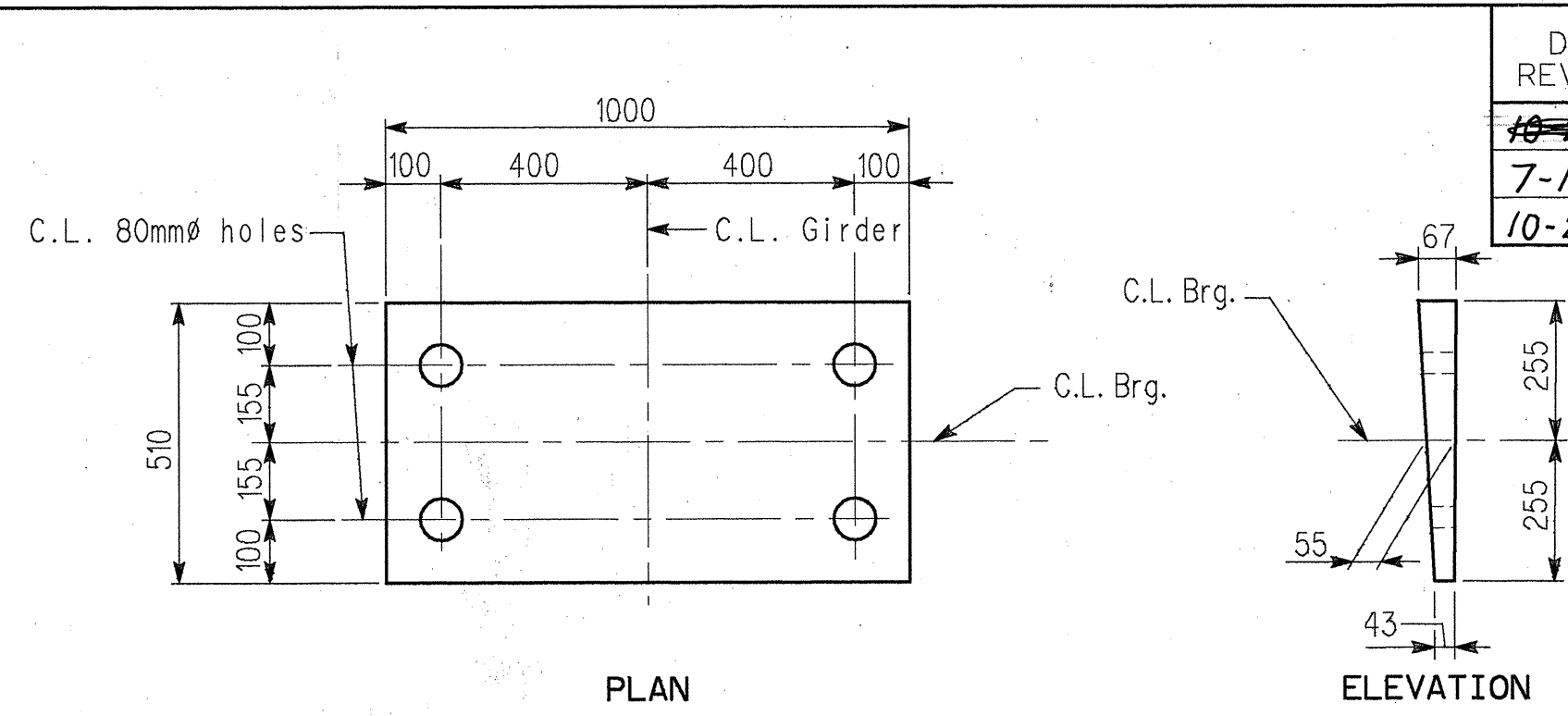
ELASTOMERIC BEARING PAD
(Bent 2,3 & 4)



ELASTOMERIC BEARING PAD
(Bent 5)

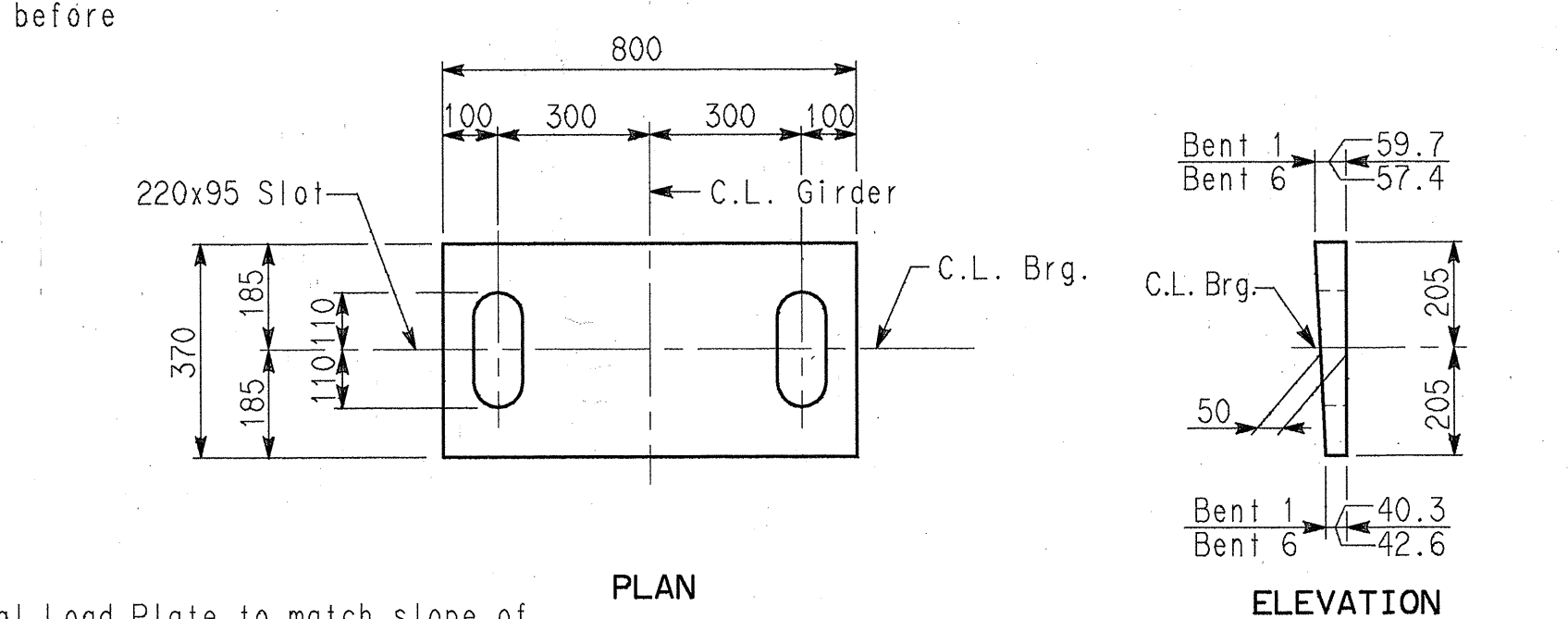


ELASTOMERIC BEARING PAD
(Bent 5)



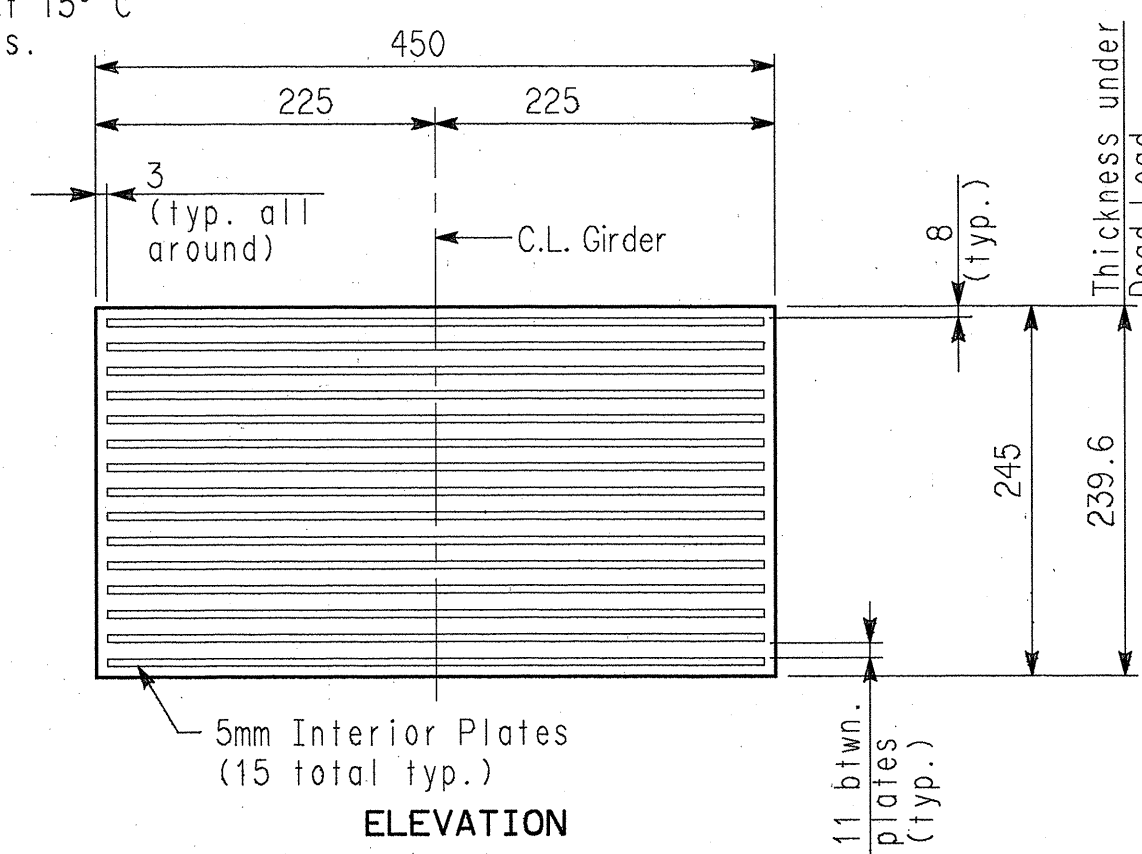
PLAN

EXTERNAL LOAD PLATE
(Bent 2,3 & 4)

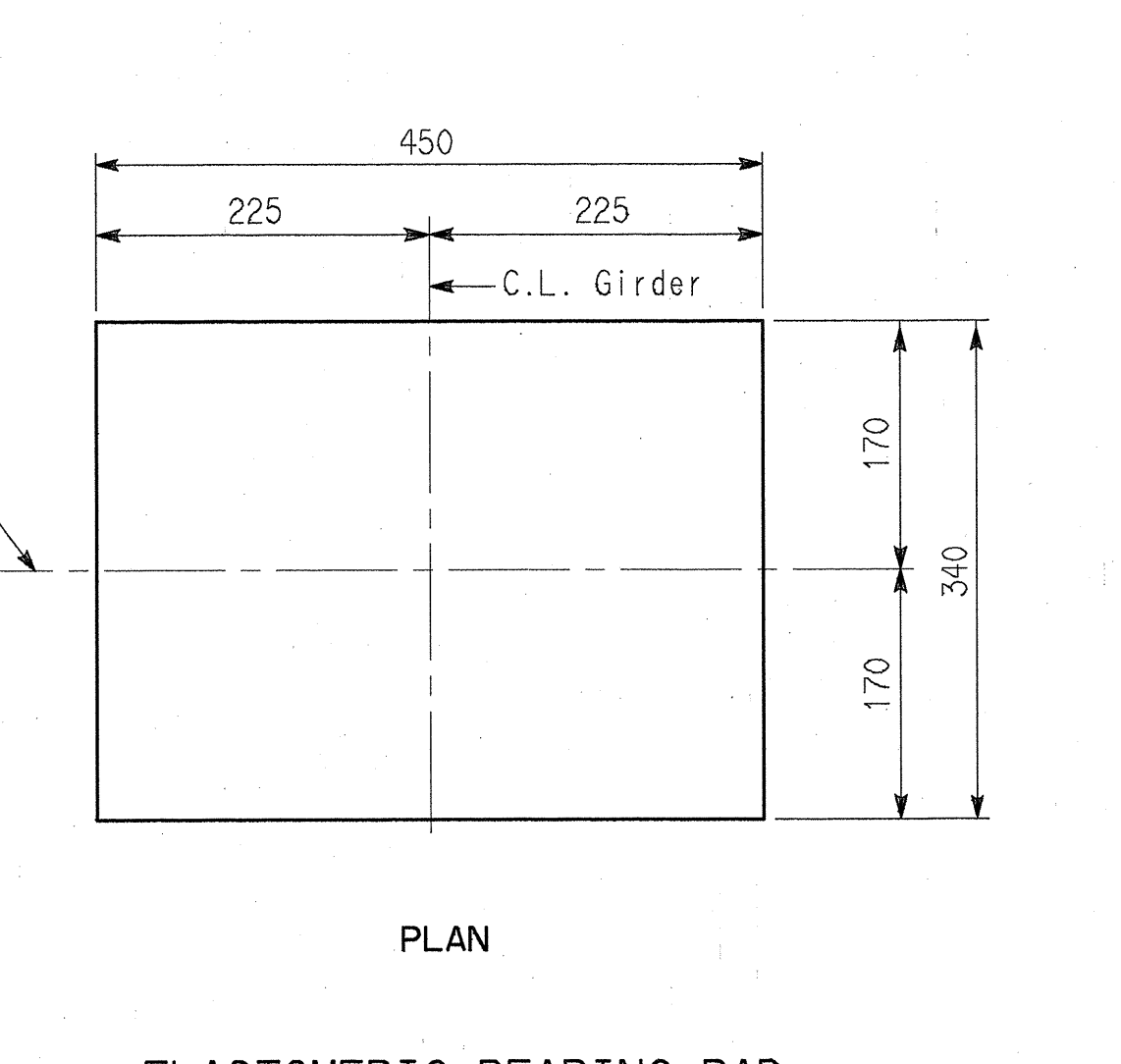


PLAN

EXTERNAL LOAD PLATE
(Bent 1 & 6)



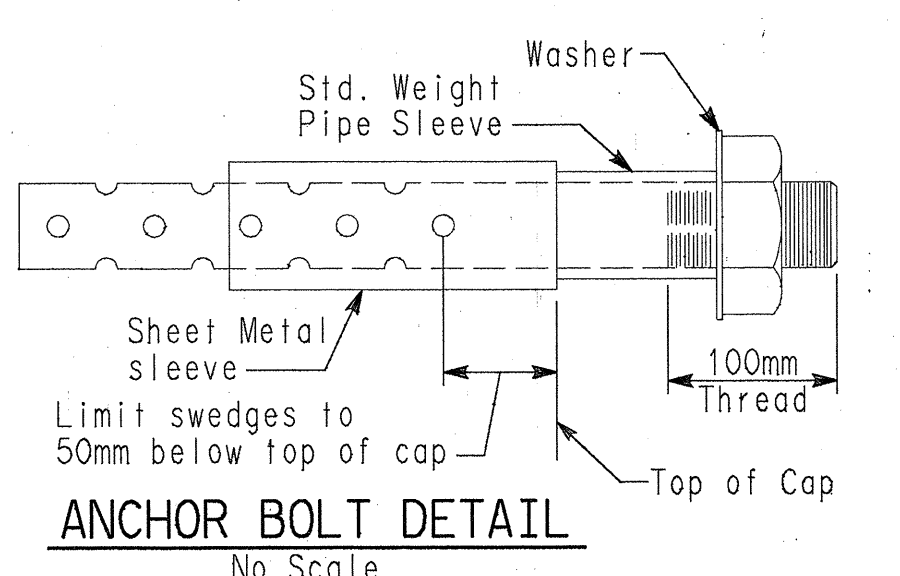
ELASTOMERIC BEARING PAD
(Bent 1 & 6)



ELASTOMERIC BEARING PAD
(Bent 1 & 6)

Note: Anchor bolts may be cast in place or drilled and grouted into place. If Anchor bolts are to be drilled and grouted into place, the 100mmØx290mm Galvanized Sheet Metal sleeve shall be cast in place as shown. It shall be dry packed with styrofoam or urethane foam or approved equal prior to pouring concrete. After pouring of the cap and prior to erection of Structural Steel, the dry pack shall be removed and holes for the anchor bolts shall be accurately drilled into the masonry. The bolts shall then be set and fixed with Portland Cement grout or an approved non-shrink gout, completely filling the holes.
If anchor bolts are to be cast in place, the 100mmØx290mm Galvanized Sheet Metal Sleeve will not be required. Galvanized Sheet metal Sleeves are to be considered subsidiary to the item "Structural Steel in Girder Span (M270, Gr. 345W)"

EXTERNAL LOAD PLATE
(Bent 5)



NOTES

Anchor bolts, Washers and Nuts shall be in accordance with subsection 807.07 of the specifications and shall be paid for in the unit price bid for "Structural Steel in Plate Girder Spans (M270, Gr. 345W)". Indentions shall be circular with rounded bottoms and staggered as shown in the details. Anchor bolts shall be Grade 105. If nuts conform to AASHTO M291 M, they shall be Property Class 8S or better, and Anchor Bolt threads shall be in accordance with ANSI B.18.2.4.6M.

Pipe sleeves shall be ASTM A53, Grade B and shall be galvanized to conform to AASHTO M232, Class C, or AASHTO M298, Grade 50.

External Load Plates shall be AASHTO M270 345W Steel. External Load Plates will not be paid for directly, but will be considered as part of the item "Elastomeric Bearings".

AASHTO M270 External Load Plates shall be cleaned in accordance with Subsection 807.84(e).

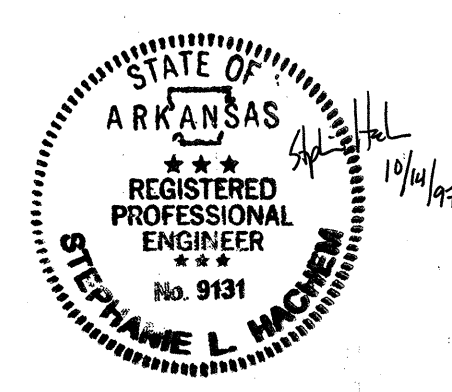
Elastomeric Pads shall conform to Section 808 for the Standard Specifications and shall be paid for at the unit price bid for "Elastomeric Bearings". The elastomeric pads must be vulcanized to the external load plate.

Interior Plated shall be in accordance with subsection 808.02 of the standard specifications.

External load plates with shear blocks shall be completely fabricated (including bevel, bolt holes and all shop welding) and shall be blast cleaned to remove rust, loose mill scale, dirt, oil, grease and other foreign substances before vulcanizing to the elastomeric bearing. The surface in contact with the elastomeric bearing shall be blast cleaned to the surface finish specified in subsection 808.04(b). Other surfaces shall be blast cleaned in accordance with subsection 807.84(b) for painted steel and 807.84(e) for unpainted grade 345W steel.

WORKING LOADS ON BEARINGS

BENT	INTERIOR	EXTERIOR
1 & 6	885Kn	788Kn
2 & 5	2755Kn	2484Kn
3 & 4	2716Kn	2450Kn



Anchor bolts, shear blocks, sleeves, loads revised MEC 10-23-97

All dimensions are in millimeters (mm) unless otherwise noted.

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EASTBOUND BRIDGE
SHEET 1 OF 1
BEARING PADS

US. HWY. 412
ARKANSAS STATE HIGHWAY COMMISSION
LITTLE ROCK, ARKANSAS

BRIDGE NO. B6686
DRAWING NO. ~~39263~~ 39263

DRAWN BY: TBI
CHECKED BY: SLH
DESIGNED BY: CLN

DATE: 10/97
DATE: 10/97
DATE: 10/97

SCALE: 1:10